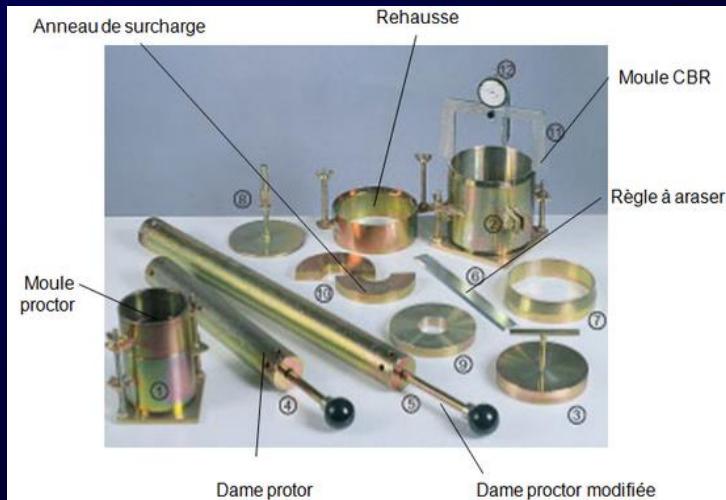
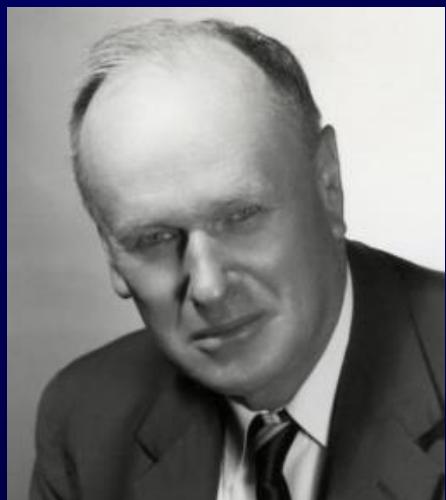


# 1<sup>ST</sup> RALPH PROCTOR LECTURE OF ISSMGE - 2016

## Railroad Performance with Special Reference to Ballast and Substructure Characteristics



Ralph Roscoe Proctor  
1894-1962

Prof. Buddhima Indraratna

*Distinguished Professor of Civil Engineering & Research Director,  
Centre for Geomechanics & Railway engineering  
(c/o ARC Centre of Excellence in Geotechnical Sciences and Engineering)  
Faculty of Engineering, University of Wollongong, NSW, Australia*

# Rail Geotechnics in a Nutshell – Key Themes

1

## Basics of track substructures and rail embankments

Indraratna et al., 2011b; Iwnicki, 2006; Li et al., 2015; Miura et al., 1998; Mundrey, 2009; Selig & Waters, 1994

3

## Experimental studies on ballast: deformation and degradation

Aursudkij et al., 2009; Brown et al., 2007; Chen et al., 2014b; Correia et al., 2007b; Indraratna et al., 1998, 2005, 2014f; Ishikawa et al., 1997, 2011, 2014a; Kennedy et al., 2012; Lackenby et al., 2007; Le Pen & Powrie, 2011; Li & Selig, 1996; McDowell et al., 2003, 2004, 2005; Selig & Sluz, 1978; Suiker et al., 2005; Sun et al., 2016; Tutumluer et al., 2008; Woodward et al., 2014

5

## Track drainage and effects of ballast fouling

Budiono et al., 2004; Darell, 2003; Dombrow et al., 2009; Ebrahimi et al., 2012, 2014; Feldman & Nissen, 2002; Giannakos, 2010; Hesse et al., 2014; Huang et al., 2009a; Indraratna et al., 2011a, 2013b; Trinh et al., 2012; Tutumluer et al., 2008

7

## Use of impact-attenuating synthetic mats

Alves Ribeiro et al., 2015; Auersch, 2006; Dahlberg, 2010; Hanson & Singleton Jr, 2006; Indraratna et al., 2014c, 2014e; Insa et al., 2014; Johansson et al., 2008; Kaewunruen & Remennikov, 2015; Markine et al., 2011; Marschnig & Veit, 2011; Nimbalkar et al., 2012; Paixão et al., 2015; Schneider et al., 2011; Sol-Sánchez et al., 2014, 2015b, 2015a; Wan et al., 2016

9

## Subgrade performance, instability and implications on track response; Stabilisation of subgrade for railways

Alves Costa et al., 2010; Cardoso et al., 2012; Correia & Cunha, 2014; Duong et al., 2013; Farris, 1970; Fatahi et al., 2015; Indraratna et al., 2010b; Li & Selig, 1996; Liu & Xiao, 2010; Miller et al., 2000; Potter & Cameron, 2005; Preteselle et al., 2013; Read et al., 1994; Selig & Sluz, 1978

11

## Numerical modelling of track and DEM simulation

Ahmed et al., 2015; Alves Costa et al., 2010, 2012; Chen et al., 2012; Correia & Cunha, 2014; D'Aguiar et al., 2012; Ferrellec & McDowell, 2012; Huang et al., 2009b, 2010; Huang & Tutumluer, 2011; Indraratna et al., 2012a, 2014a; Lu & McDowell, 2006, 2010; McDowell et al., 2006; Ngo et al., 2014, 2015; Quinn et al., 2010; Suiker & de Borst, 2003; Tutumluer et al., 2007, 2012

13

## Specific design functions including transition zones

Coelho et al., 2011; Fernandes et al., 2012; Giner & López-Pita, 2009; Huang & Brennecke, 2013; Le Pen et al., 2014b; Li & Davis, 2005; Mishra et al., 2014a; Raymond, 1986; Varandas et al., 2014

15

## Track assessment using Image analysis

Abadi et al., 2015; Ajayi et al., 2015; Fernald, 2005; Le Pen et al., 2014a; Sun et al., 2014; Tutumluer et al., 2006, 2012

17

## Selected Practice Guides and Technical Specifications for ballasted tracks

AREMA, 2003, 2015; British Standards Institution, 2003; Canadian National Railway, 2015; German Institute for Standardisation, 2008, 2013; Indian Railway Specification, 2004; International Union of Railways, 2008; IRPWM, 2004; Japanese Standards Association, 2014; Railtrack, 2000; Standards Australia, 2015

2

## Load distribution in track, moving loads and dynamic track analysis

Choi, 2013; Correia et al., 2007a; Esveld, 2001; Ishikawa et al., 2011, 2014b; Kaewunruen & Remennikov, 2008; Momoya et al., 2005; Powrie et al., 2007; Remennikov & Kaewunruen, 2008; Yang et al., 2009

4

## Theoretical aspects and constitutive modelling of ballast and sub-ballast

Cui et al., 2013; Desai & Janardhanam, 1983; Einav, 2007a, 2007b; Indraratna et al., 2011b, 2012b, 2014b, 2014f; Knothe & Grassie, 1993; Rowe, 1962; Suiker & de Borst, 2003; Tennakoon et al., 2015; Yang et al., 2008; Zhai et al., 2004, 2009

6

## Use of geosynthetics including geogrids, geotextiles and geocells

Brown et al., 2007; Chen et al., 2014a; Dash & Shivadas, 2012; Fernandes et al., 2008; Indraratna & Nimbalkar, 2013; Indraratna et al., 2010a; 2013a; 2014e; 2015; Leshchinsky & Ling, 2013; McDowell & Stickley, 2006; Mishra et al., 2014b; Qian et al., 2015; Raymond, 1986, 2002; Tatsuoka et al., 1992, 1996, 2008; Tutumluer et al., 2012

8

## Role of sub-ballast including capping layer and structural fills

Chrismer & Davis, 2000; Fatahi et al., 2011; Fortunato et al., 2012; Haque et al., 2008; Indraratna et al., 2015; Radampola et al., 2008; Trani & Indraratna, 2010

10

## Ballast bonding (polyurethane) for improved track resiliency

Dersch et al., 2010; Jubin, 2012; Keene et al., 2012, 2014; Kennedy et al., 2013; Woodward et al., 2007, 2014

12

## Field Instrumentation and performance verification

Alves Costa et al., 2012; Choi, 2013; Indraratna et al., 2010a, 2010b, 2014d; Kaewunruen & Remennikov, 2015; Le Pen et al., 2014b; Read et al., 1994; Sánchez et al., 2014; Schneider et al., 2011; Woodward et al., 2007

14

## Aspects of track maintenance and scheduling

Ebrahimi & Keene, 2011; Ferreira & Higgins, 1998; Higgins et al., 1999; Kaewunruen et al., 2015; Marschnig & Veit, 2011; Peng et al., 2011; Quiroga & Schnieder, 2012; Thom, 2007; Woodward et al., 2007; Zhang et al., 2013

16

## Energy geotechnics and carbon footprint for track engineering

Åkerman, 2011; Chang & Kendall, 2011; Federici et al., 2008; Kaewunruen et al., 2015; Kiani et al., 2008; Schwarz, 2008; UIC, 2013, 2015; Westin & Kågeson, 2012

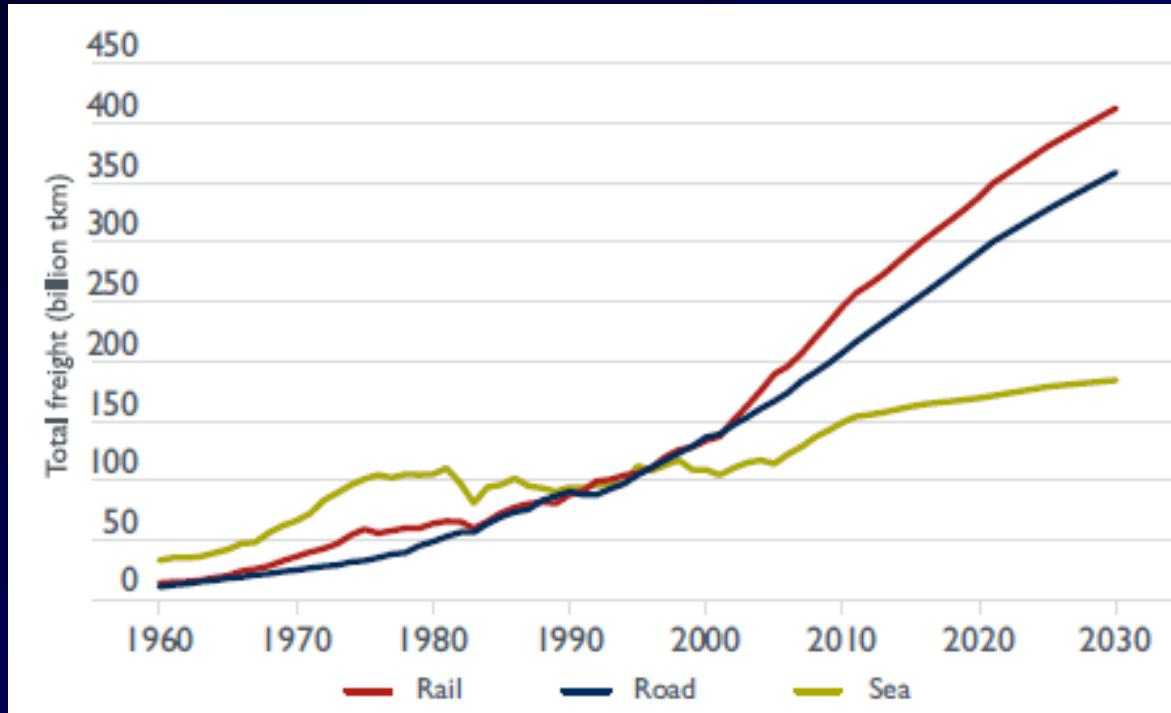
Ref: Table 1 - Indraratna (2016), 1<sup>st</sup> Proctor Lecture of ISSMGE, Transportation Geotechnics, Vol. 7: 74–114.

# Presentation Outline

- Ground Problems and Railroad Challenges
- Track Capacity for Fast Heavy Haul Demands
- Fundamental and Applied Research
- Field Applications and Performance Verification
- Industry Impact and Design Innovation

# Introduction

- Demand for freight and passenger transport has increased in the past decade.
- Large repetitive loads from traffic cause rapid degradation and deformation of tracks.
- Inclusion of resilient materials (geosynthetics & shock mats) helps to enhance track response.



Figures from "Road and rail freight: competitors or complements?" Bureau of Infrastructure, Transport and Regional Economics, Australian Government Canberra.

# Problems in Rail Track Substructure



Ballast Crushing



Subgrade Clay Pumping



Coal fouling



Void Clogging



Differential settlement (Courtesy, Prof AK Suiker)



Poor Drainage

# Track Buckling due to Insufficient Lateral (confining) pressure



# Requirements for Heavy Haul Fast Tracks

1. Ballast: Reduced Degradation and lateral Movement for greater longevity.
2. Sub-ballast: Improved filtration and drainage under large cyclic loads.
3. Foundation soil (subgrade): Increased shear strength and reduced settlement.
4. Rail and Sleepers – Minimise Impact Damage at high speeds and axle loads

## OUTCOME: New Standards, Design Innovation and Construction Alternatives.



**Placing of synthetic energy absorbing mats (SEAM)**



Use of geosynthetics in track for improved resiliency, better drainage and reduced deformation.



# Large-scale Cyclic Triaxial Rigs Built at UoW



Prismoidal Triaxial Rig to  
Simulate a Track Section  
(Specimen: 800x600x600 mm)

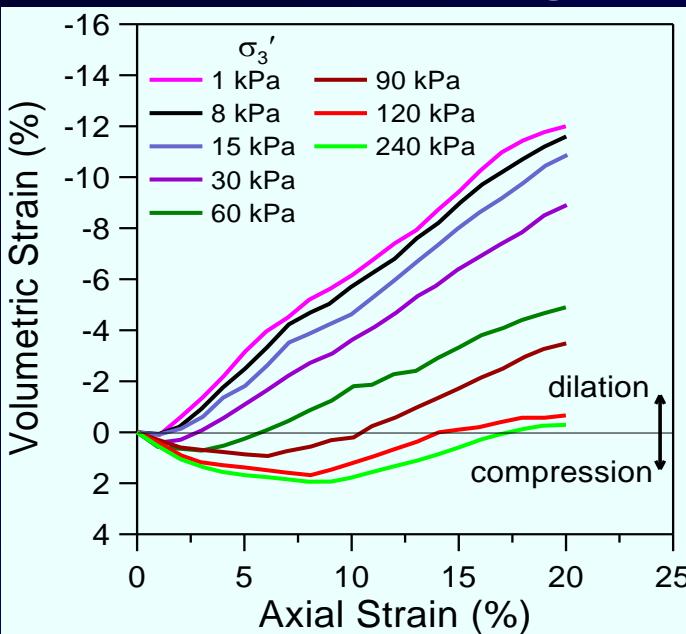


Cylindrical Triaxial Equipment  
(Specimen: 300 mm dia.x600 mm high)

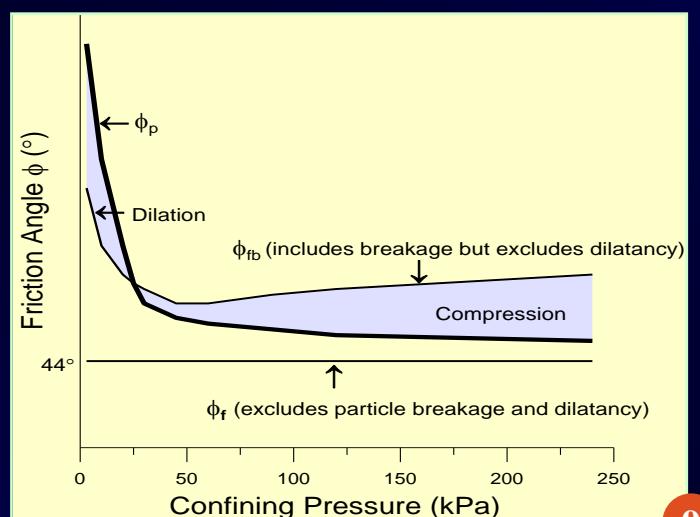
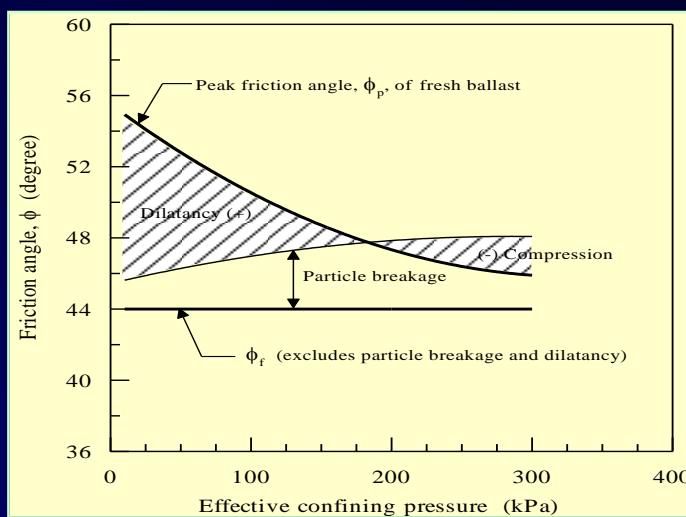
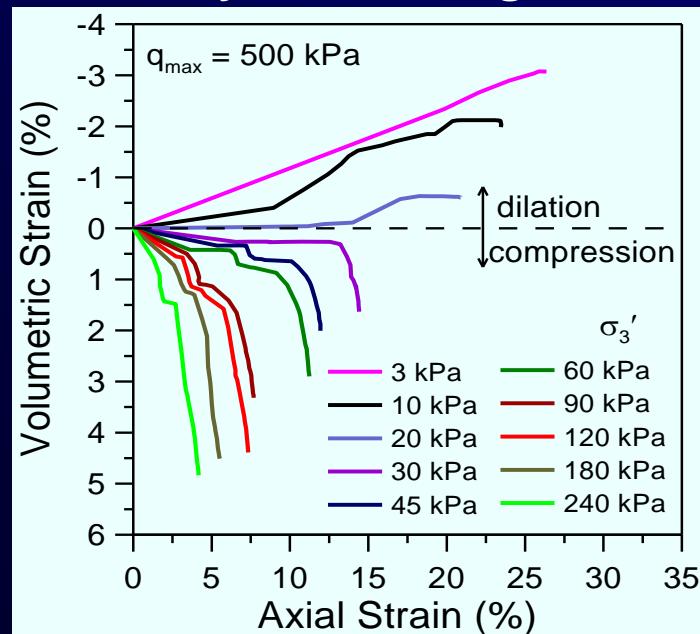
# Effect of Confining Pressure on Strain Behaviour of Ballast

Indraratna, Lackenby and Christie (2005), Geotechnique

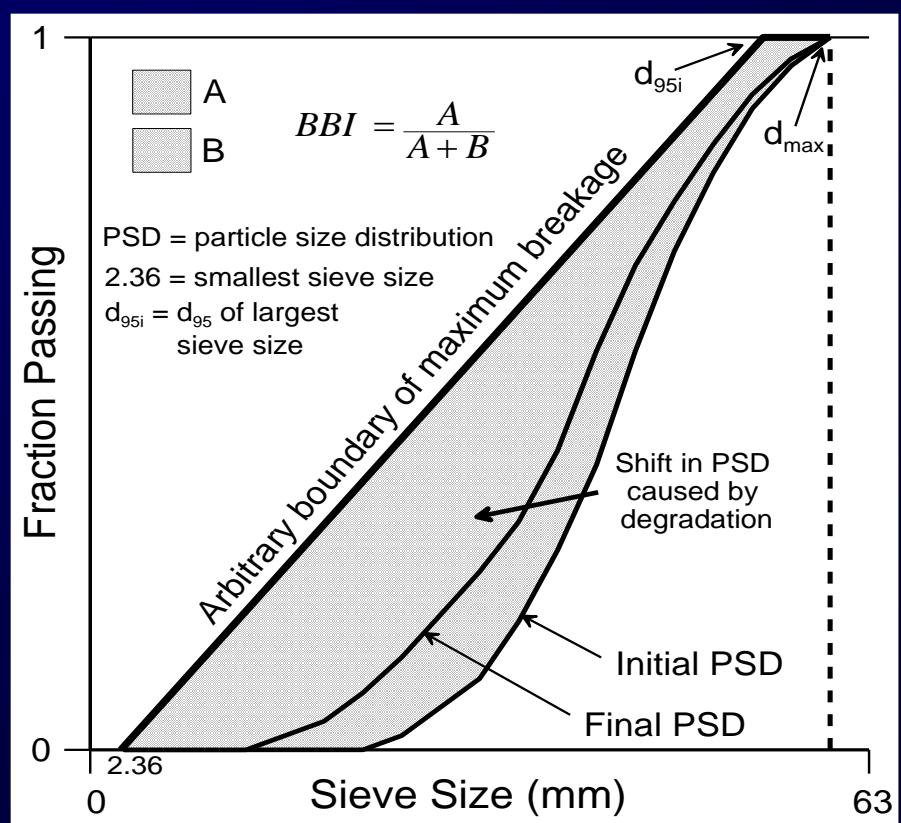
## Monotonic Loading



## Cyclic Loading



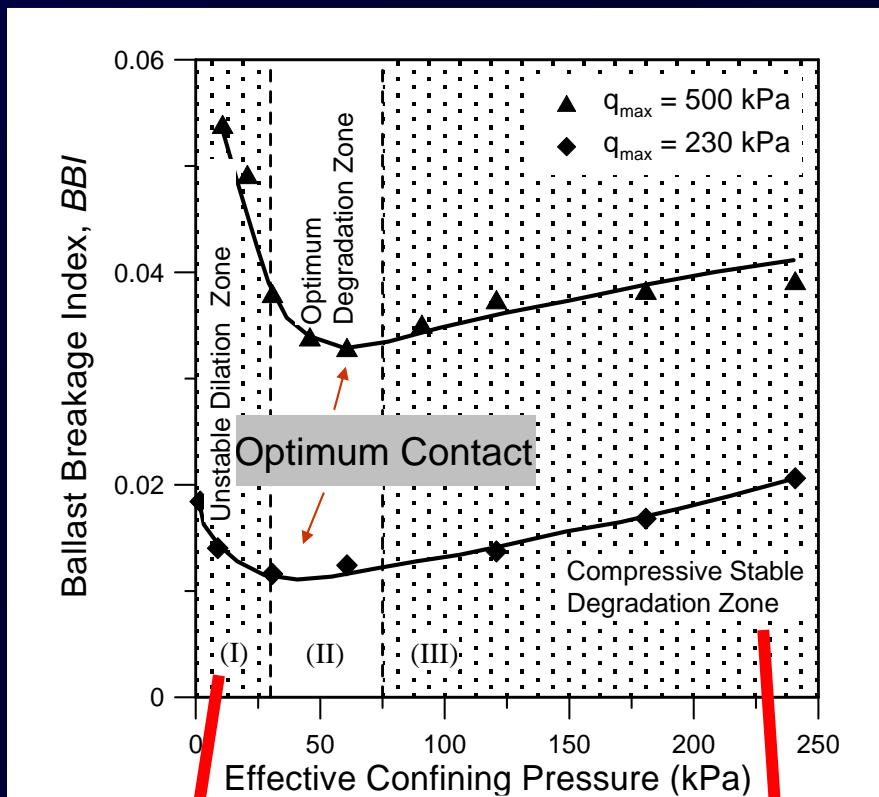
# Effect of Confining Pressure on Particle Degradation (Cyclic Loading)



## Ballast Breakage Index (BBI)

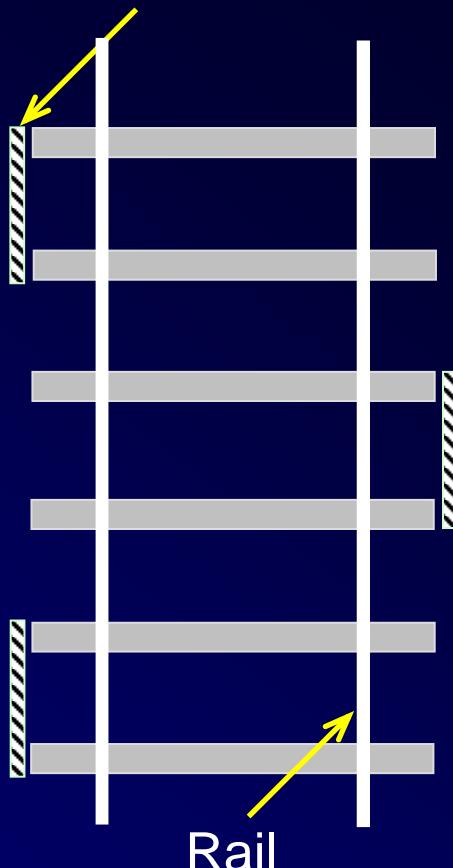
Indraratna, Lackenby and Christie (2005)

Vol. 55(4), Geotechnique, ICE, UK.

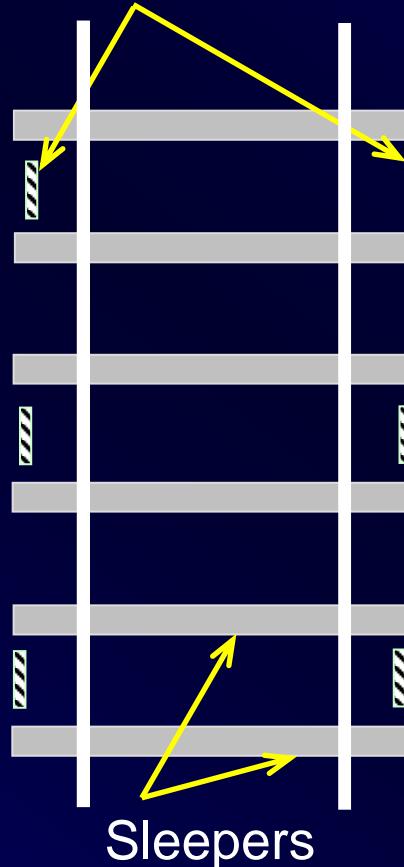


# Increasing Confining Pressure using: Intermittent Lateral Restraints or Embedded Winged Sleepers

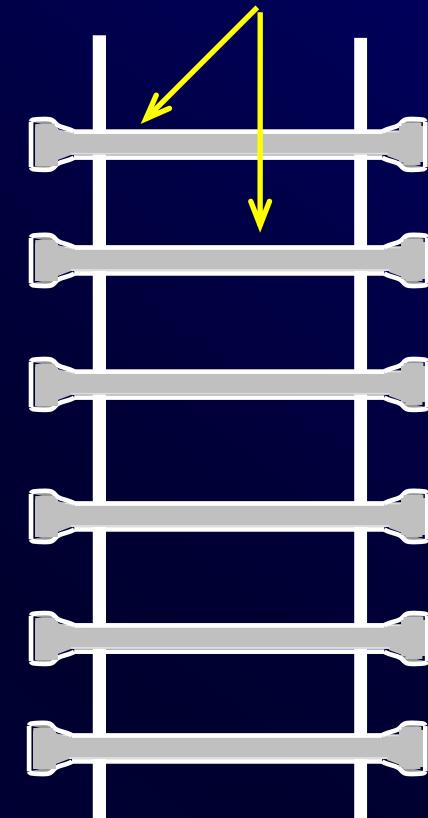
Intermittent lateral restraints



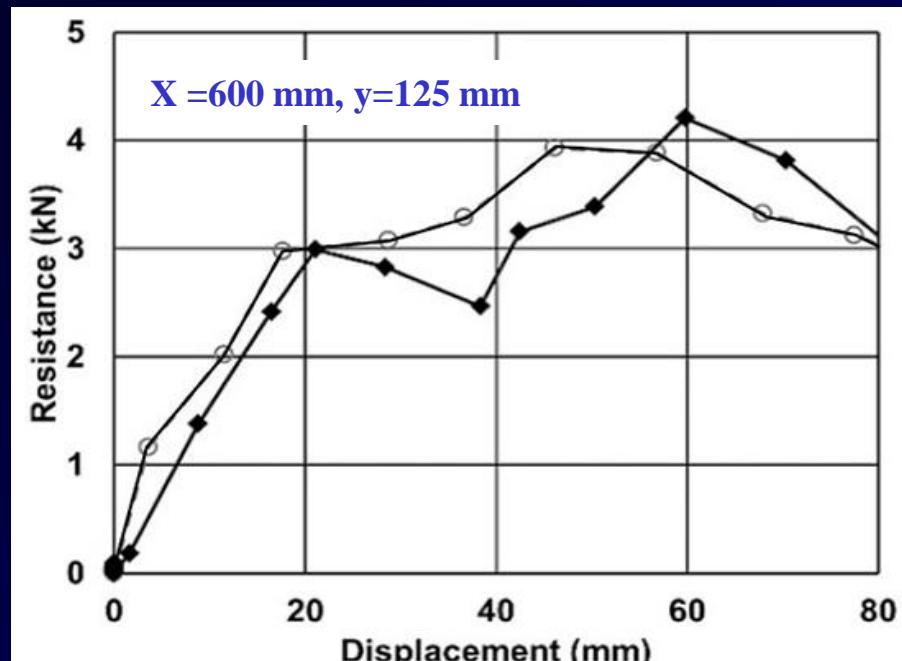
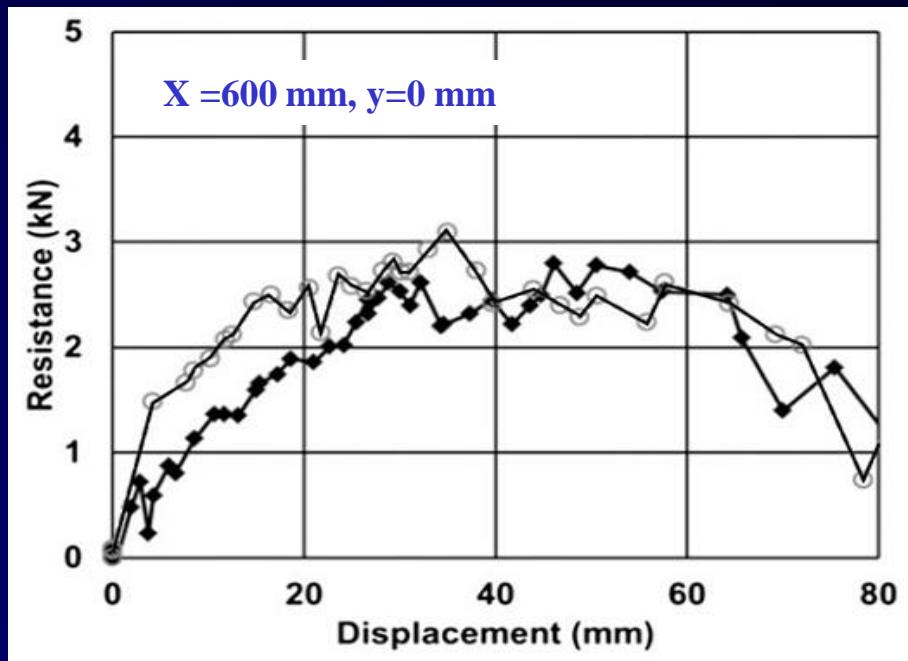
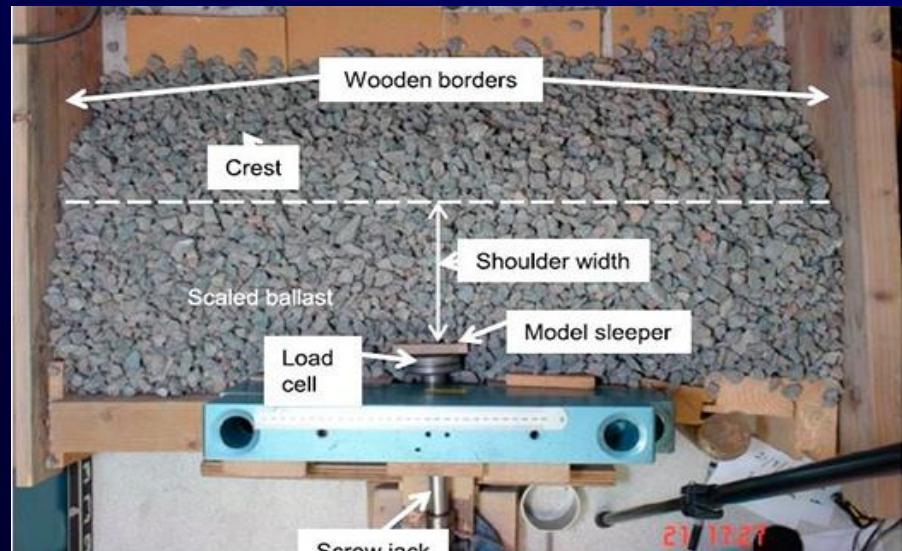
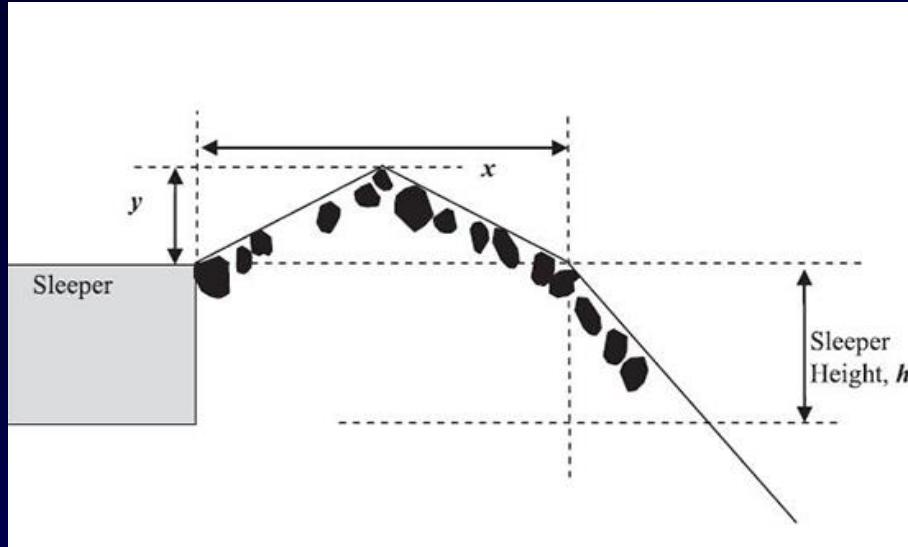
Lateral restraints



Winged sleepers



# Lateral Resistance Offered by Shoulder Ballast

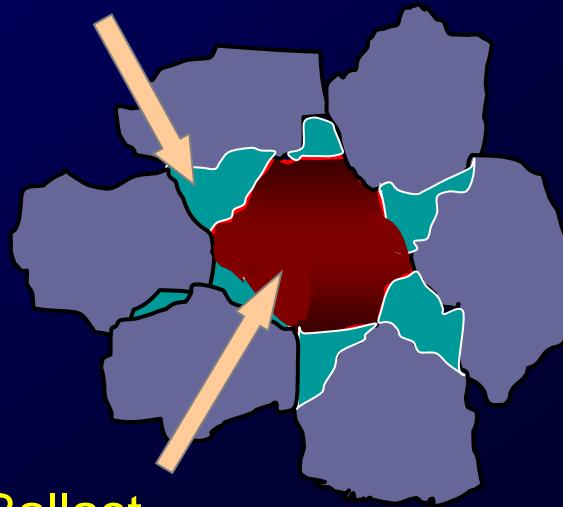


# Preventing Particle Breakage – Computational Modelling

(Salim and Indraratna, 2000; Canadian Geotechnical Journal)

Before Loading

Voids



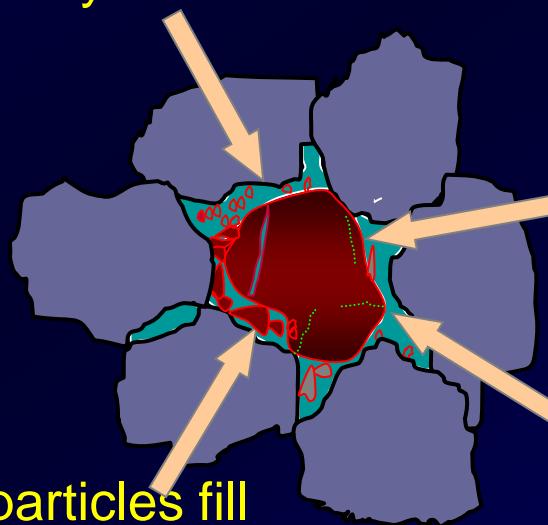
After Loading

Asperity wear

Broken particles fill  
voids (fouling)

New hairline  
micro-cracks

Sharp corners  
broken off



$$d\varepsilon_s^p = \frac{2\alpha\kappa \left( \frac{p}{P_{cs}} \right) \left( 1 - \frac{P_{o(i)}}{P_{cs(i)}} \right) (9 + 3M - 2\eta * M) \eta d\eta}{M^2 (1 + e_i) \left( \frac{2P_o}{p} - 1 \right) \left[ 9(M - \eta *) + \frac{B}{p} \{ \chi + \mu(M - \eta *) \} \right]}$$

$$\frac{d\varepsilon_v^p}{d\varepsilon_s^p} = \frac{9(M - \eta)}{9 + 3M - 2\eta * M} + \left( \frac{B}{p} \right) \left[ \frac{\chi + \mu(M - \eta *)}{9 + 3M - 2\eta * M} \right]$$

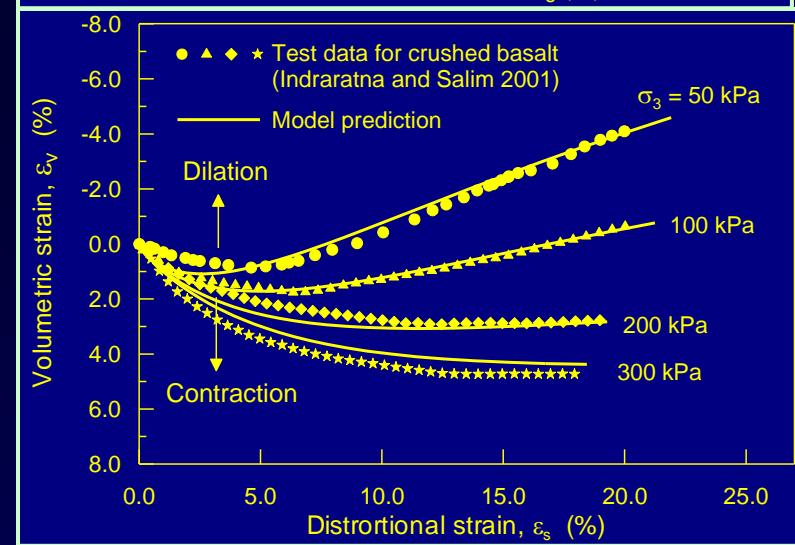
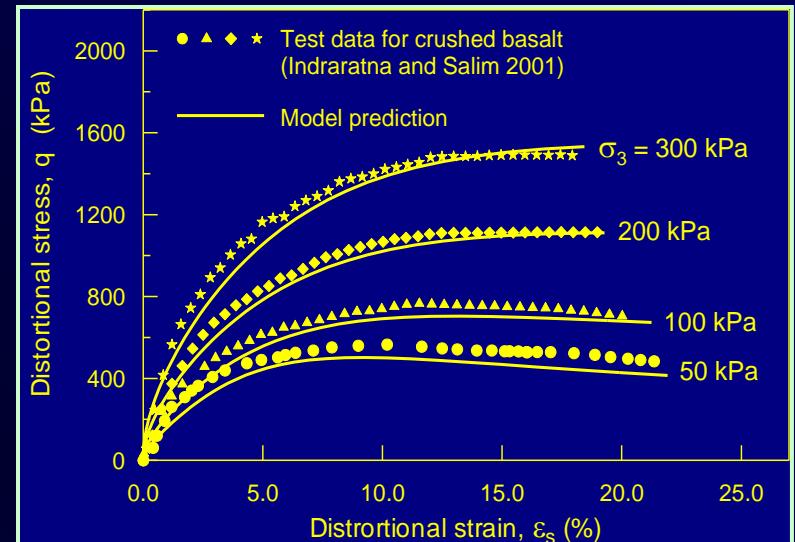
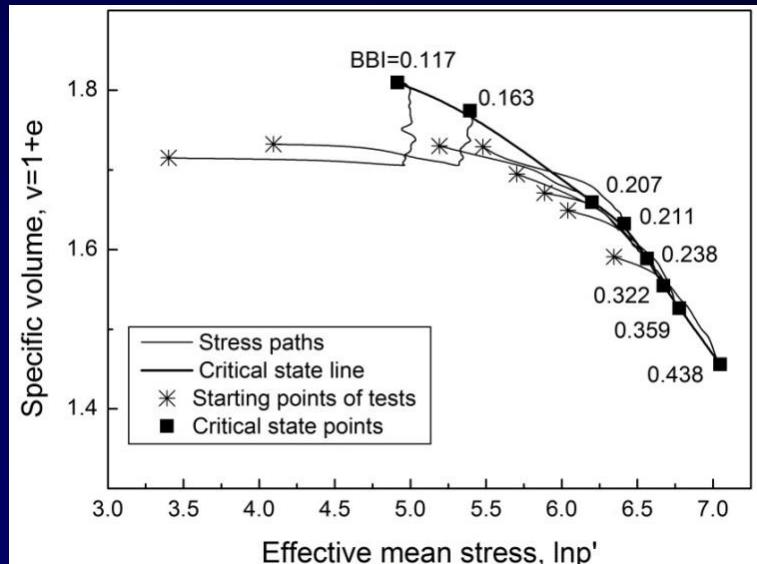
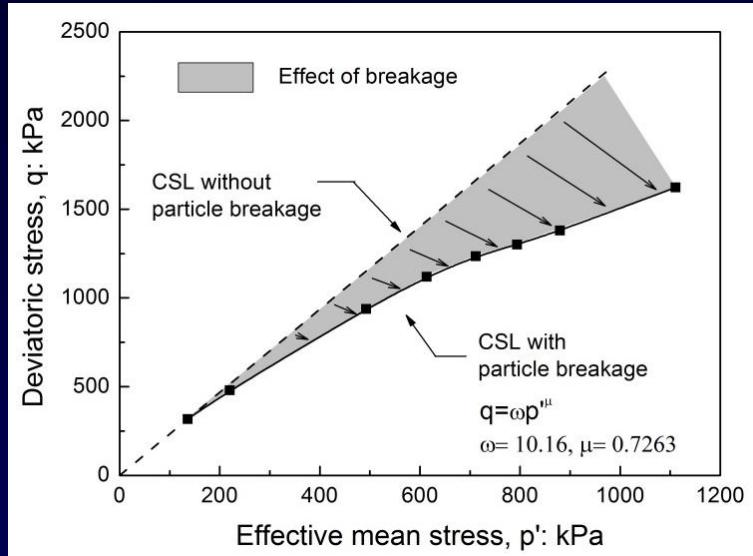


# Constitutive model: Critical State capturing particle breakage

Indraratna, Sun & Nimbalkar (2014), Canadian Geotechnical Journal, Vol. 52(1), 73-86

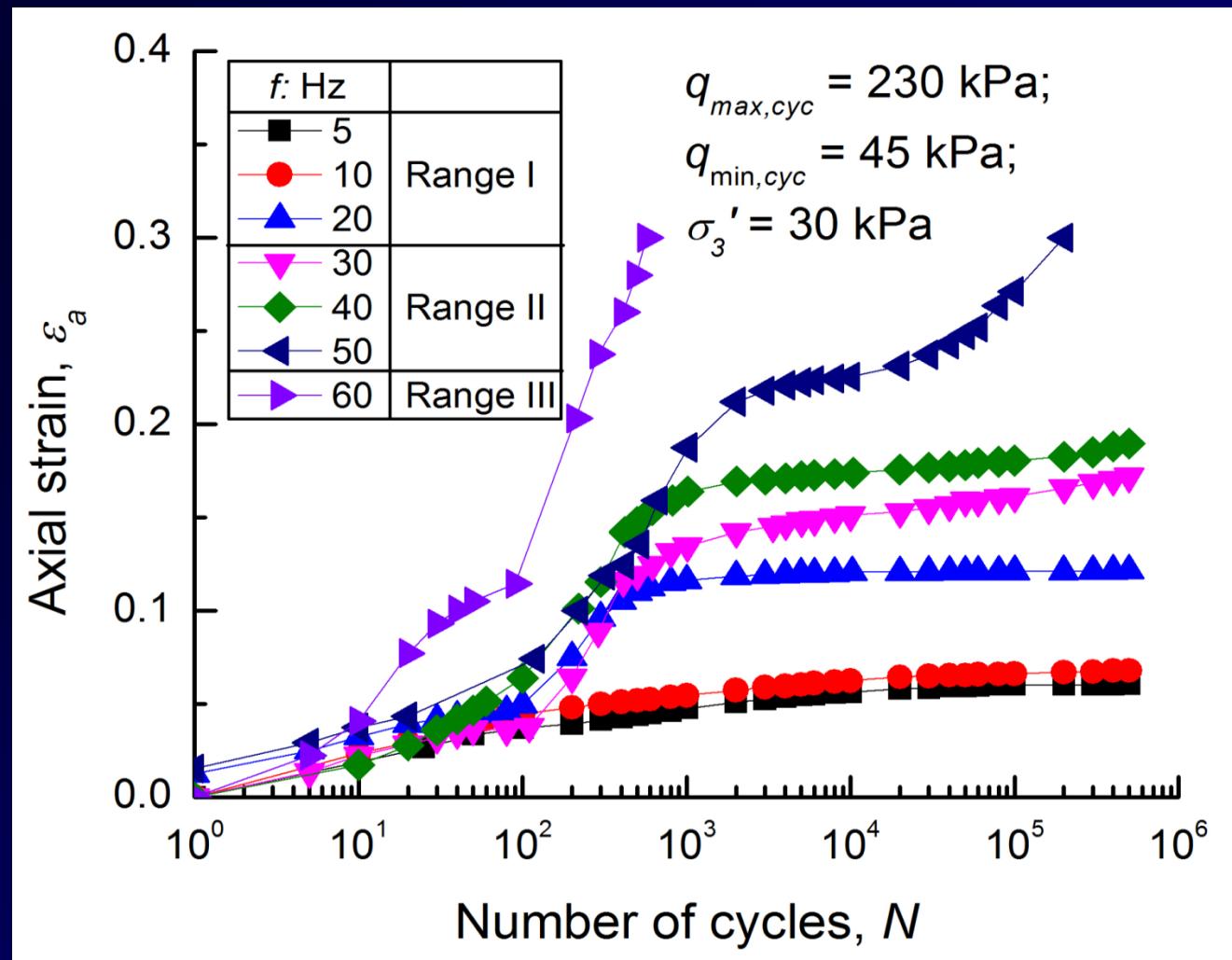
$$M_c = M_{c0} - [1 - \exp(-\alpha \cdot BBI)]$$

$M_{c0}$  is critical state stress ratio for  $BBI = 0$



# Effect of frequency on the axial strain of ballast

Sun, Q., Indraratna, B. & Nimbalkar, S. (2014). Géotechnique, Vol. 64(9), 746-751.



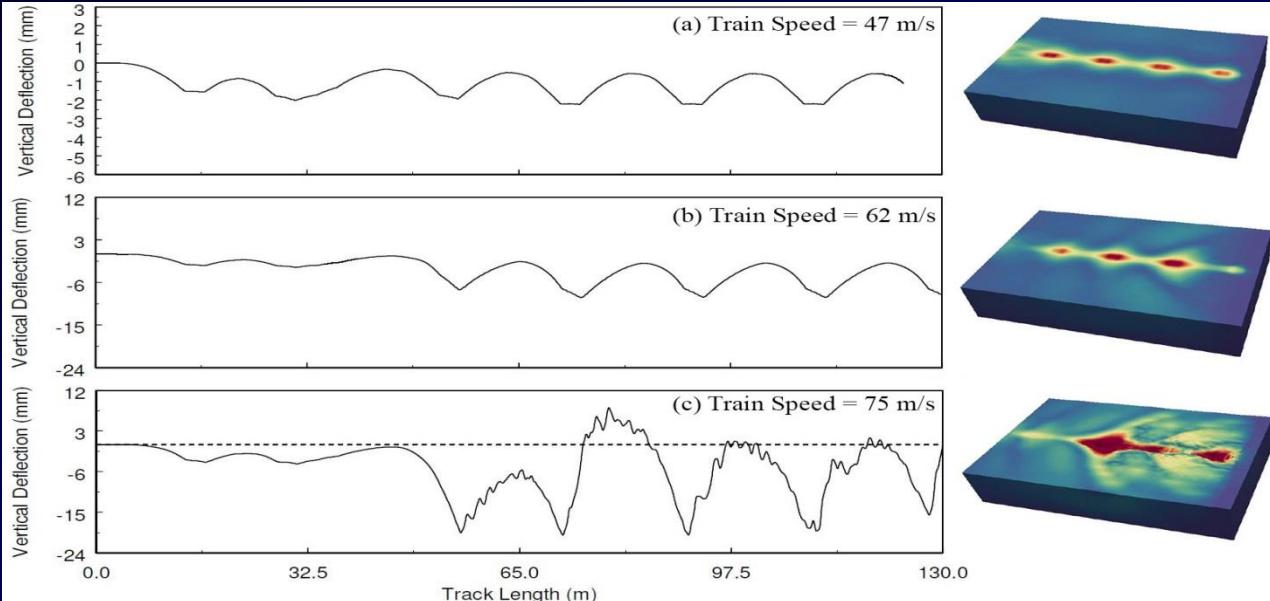
Range I: Plastic shakedown ( $5 \text{ Hz} \leq f \leq 20 \text{ Hz}$ )

Range II: Plastic shakedown followed by Ratcheting ( $30 \text{ Hz} \leq f \leq 50 \text{ Hz}$ )

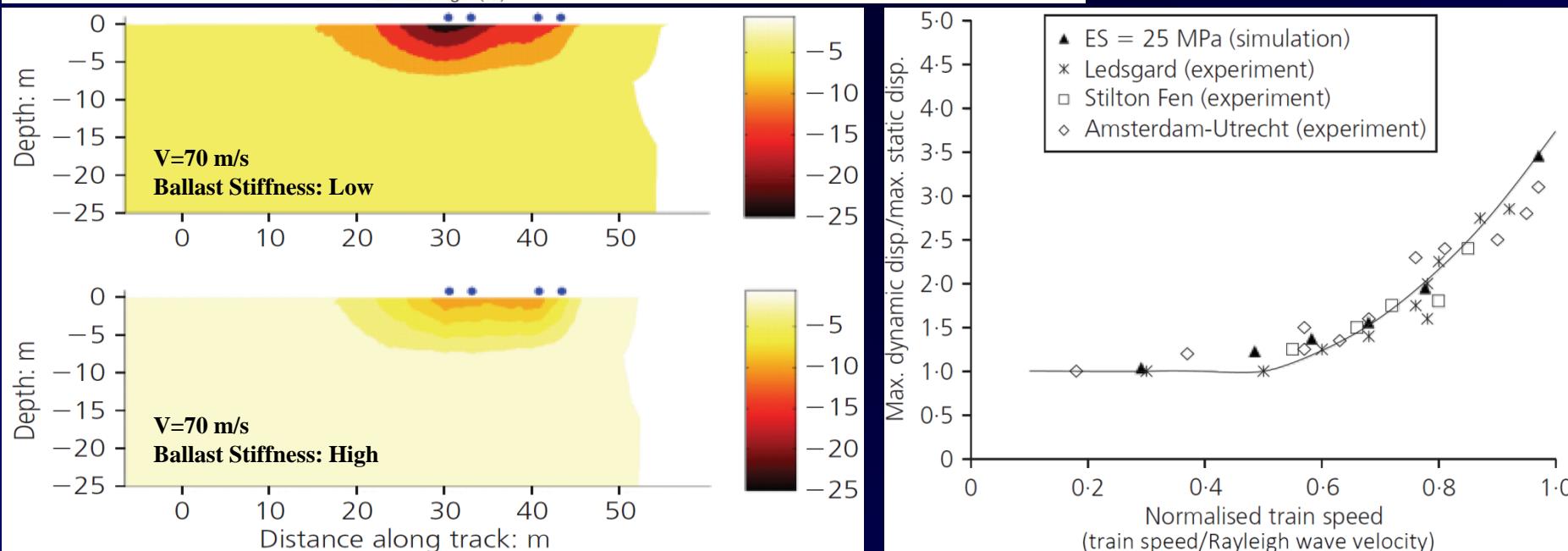
Range III: Plastic collapse ( $f = 60 \text{ Hz}$ )

# Dynamic Track Analysis and Substructure Response

Transient vertical deflection of typical sleeper and development of the ground Mach Cone



Woodward, Lagrouche, and El-Kacimi (2013). *11th International Conference on Vibration Problems*, Lisbon, Portugal.



# Impact loading that leads to track damage

Different wheel and rail irregularities contribute to Impact loading

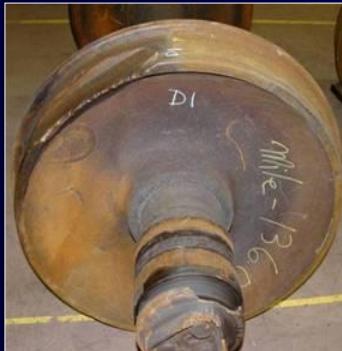
Worn wheel surface



Worn rail surface



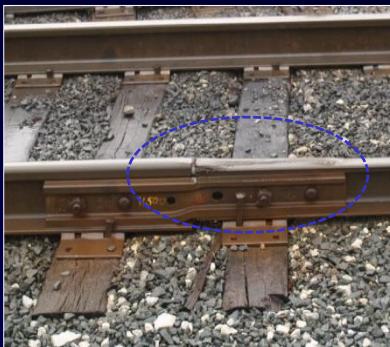
Wheel flats



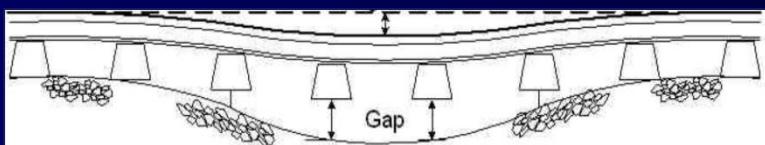
Rail corrugation



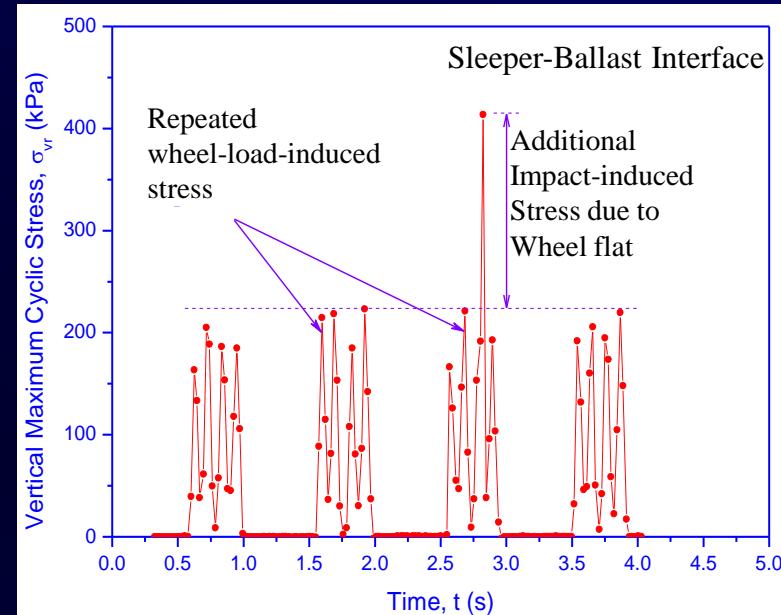
Bad welds, joints and switches



Unsupported sleepers



## Field Measurements



Cyclic stresses transmitted to the ballast by coal train with 100 ton wagons having wheel irregularities

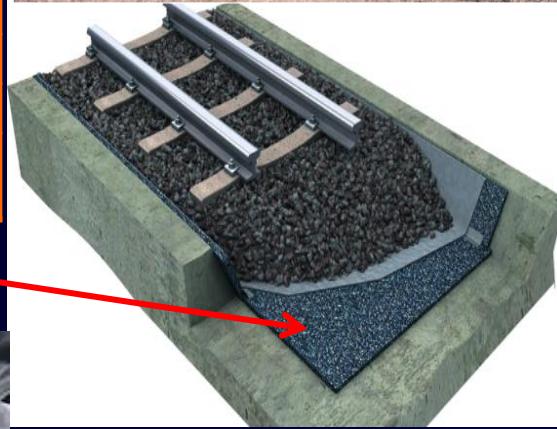
Indraratna et al. (2010). JGGE, ASCE

# Use of Energy Absorbing Rubber Mats to Prevent Impact Damage

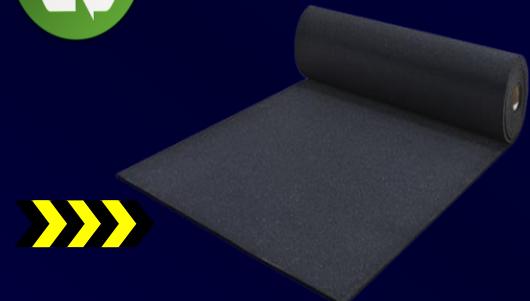


Subgrade type	Location of shock mat	Ballast Breakage Index (BBI)
<b>Without shock mat</b>		
<b>Stiff</b>	-	<b>0.170</b>
<b>Soft</b>	-	<b>0.080</b>
<b>With Shock mat</b>		
<b>Stiff</b>	<b>Above ballast</b>	<b>0.145</b>
<b>Stiff</b>	<b>Below ballast</b>	<b>0.129</b>
<b>Stiff</b>	<b>Above &amp; below ballast</b>	<b>0.091</b>
<b>Soft</b>	<b>Above ballast</b>	<b>0.055</b>
<b>Soft</b>	<b>Below ballast</b>	<b>0.056</b>
<b>Soft</b>	<b>Above &amp; below ballast</b>	<b>0.028</b>

Shock Mat

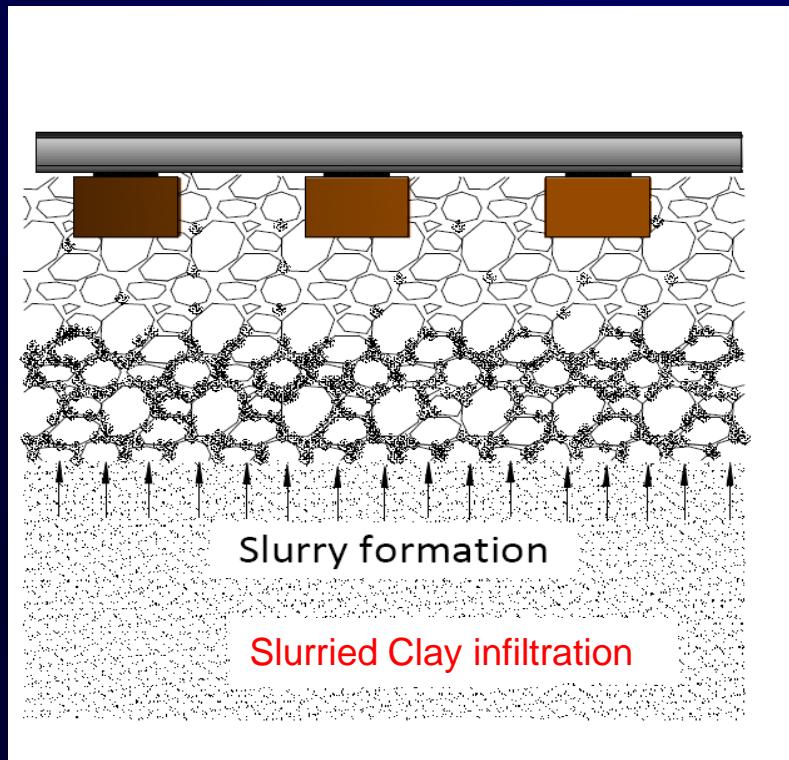
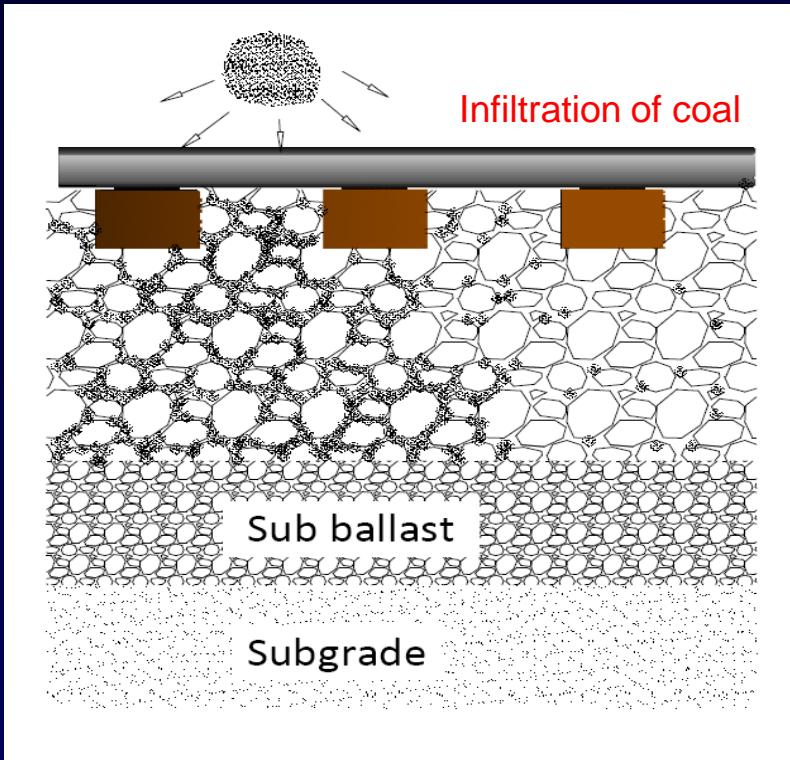


WASTE TURNS  
INTO VALUE



Nimbalkar, Indraratna, Dash & Christie (2012). JGGE, ASCE, 138(3): 281-294.

# Role of Ballast Fouling on Track Performance



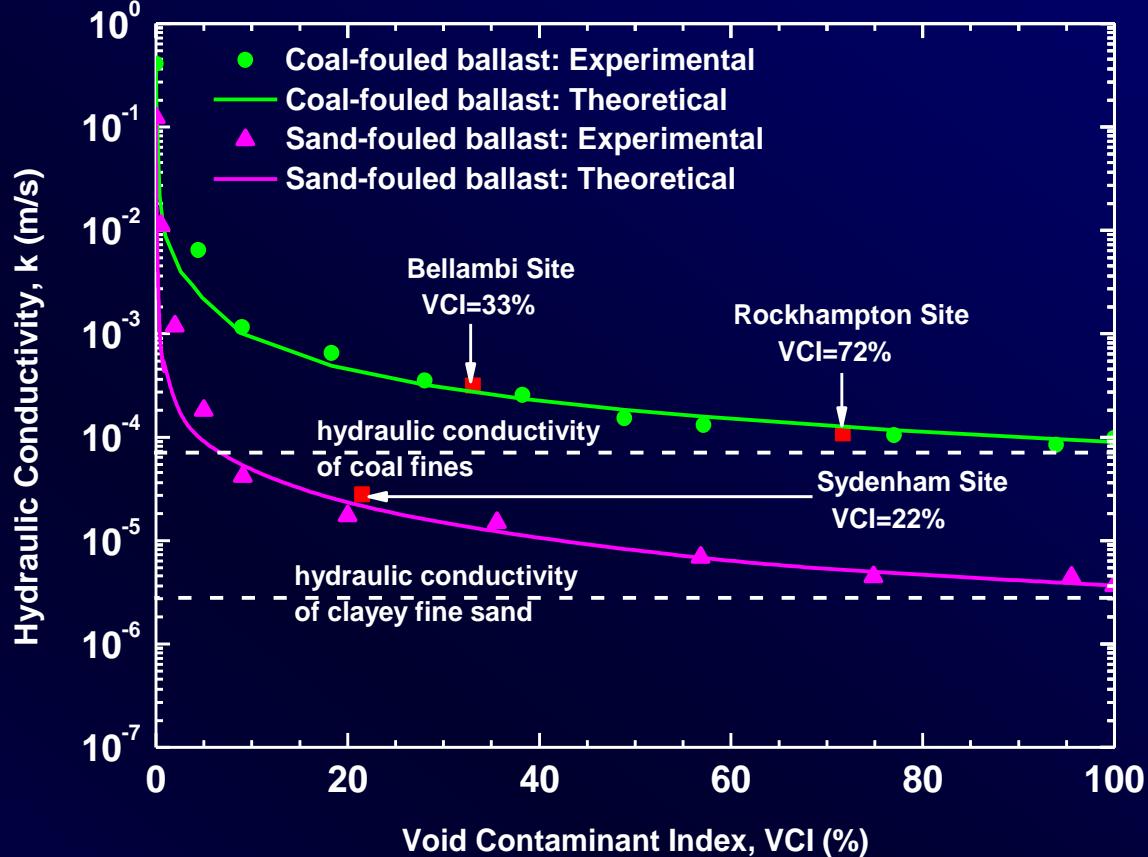
Void Contaminant Index (VCI) proposed by UOW

$$VCI = \frac{(1+e_f)}{e_b} \times \frac{G_{s,b}}{G_{s,f}} \times \frac{M_f}{M_b} \times 100$$

Tennakoon, Indraratna, Cholachat, Nimbalkar and Neville  
(2012) ASTM Geotechnical Testing Journal, 35(4): 1-12

- $e_b$  = Void ratio of clean ballast
- $e_f$  = Void ratio of fouling material
- $G_{s-b}$  = Specific gravity of clean ballast
- $G_{s-f}$  = Specific gravity of fouling material
- $M_b$  = Dry mass of clean ballast
- $M_f$  = Dry mass of fouling material

# Impeded Track Drainage due to Ballast Contamination



Large-scale permeability test apparatus

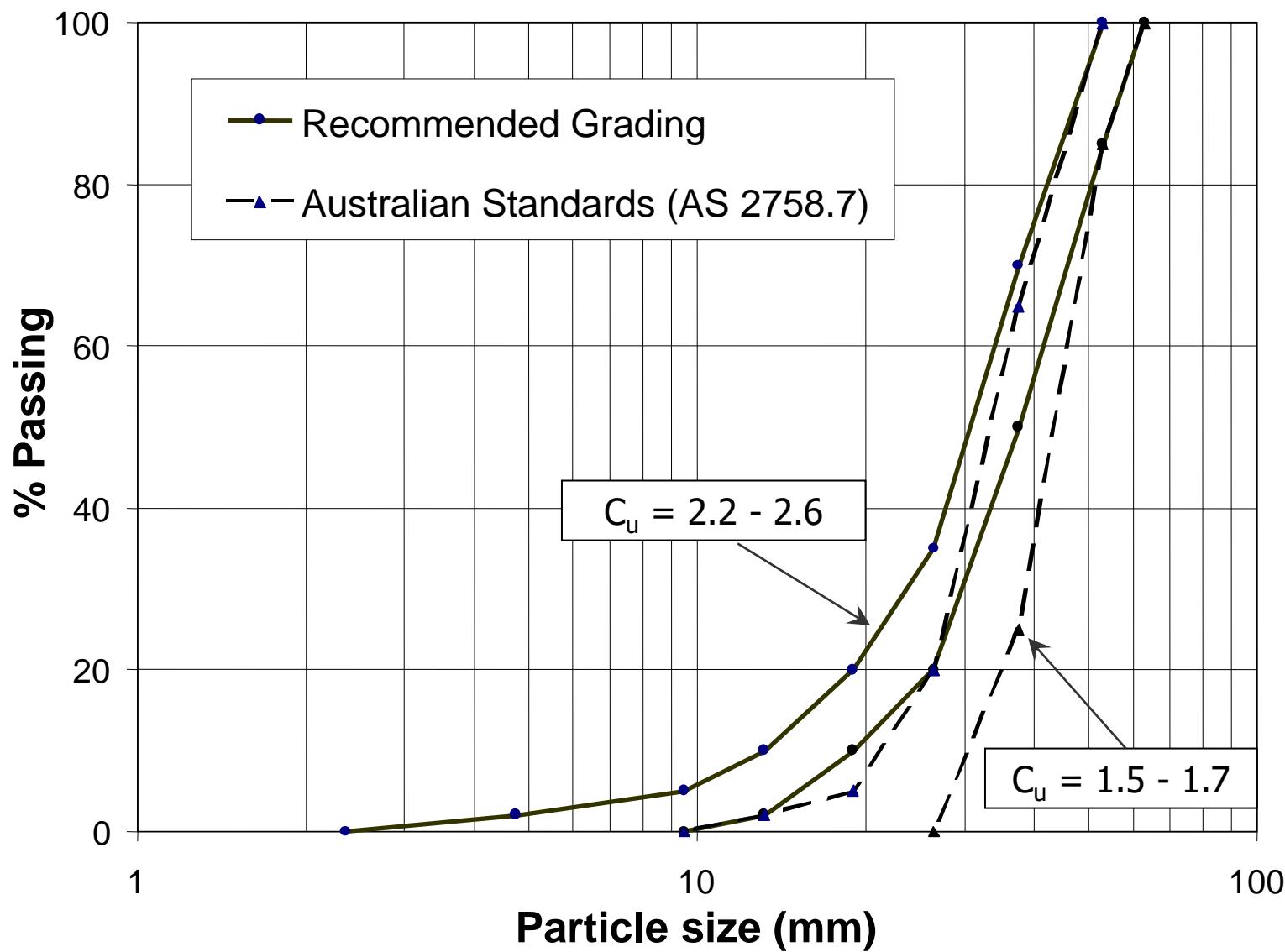
Hydraulic Conductivity ( $k$ ) of fouled ballast

$$k = \frac{k_b \times k_f}{k_f + VCI / 100 \times (k_b - k_f)}$$

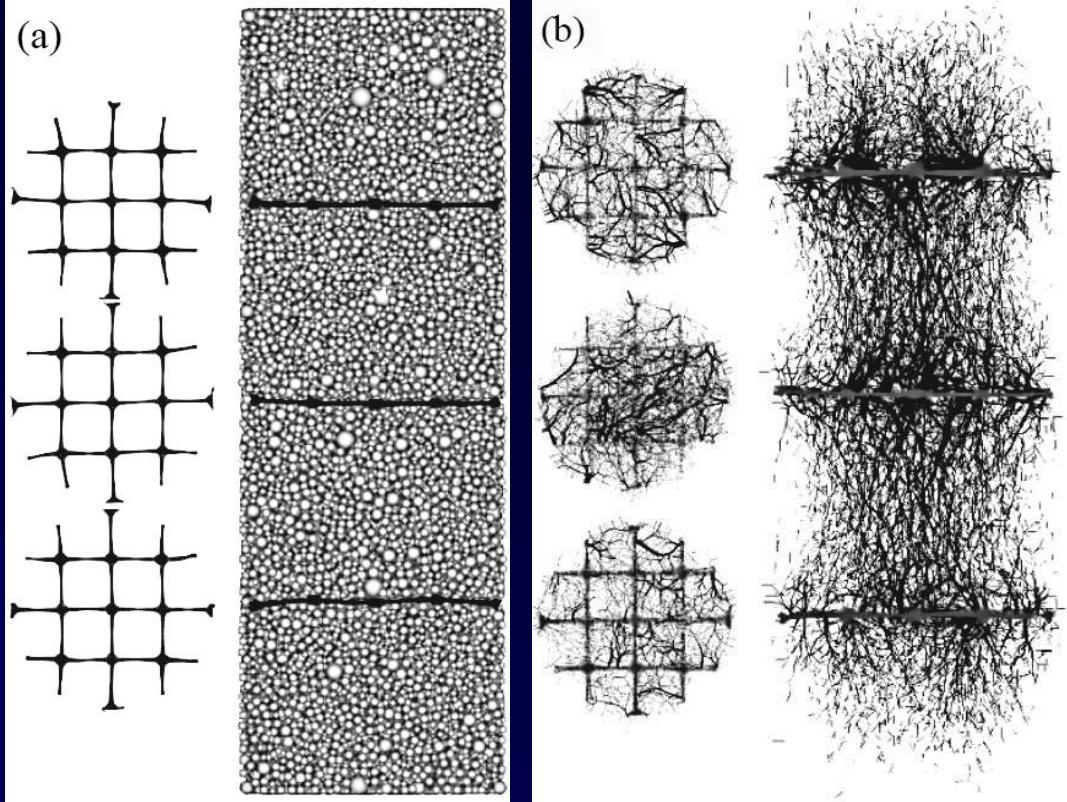
$k_b$  = Hydraulic conductivity of clean ballast

$k_f$  = Hydraulic conductivity of fouling material

# Recommended New Railway Ballast Grading



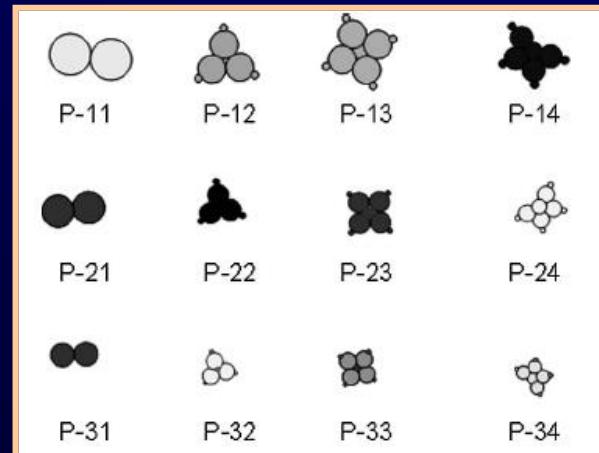
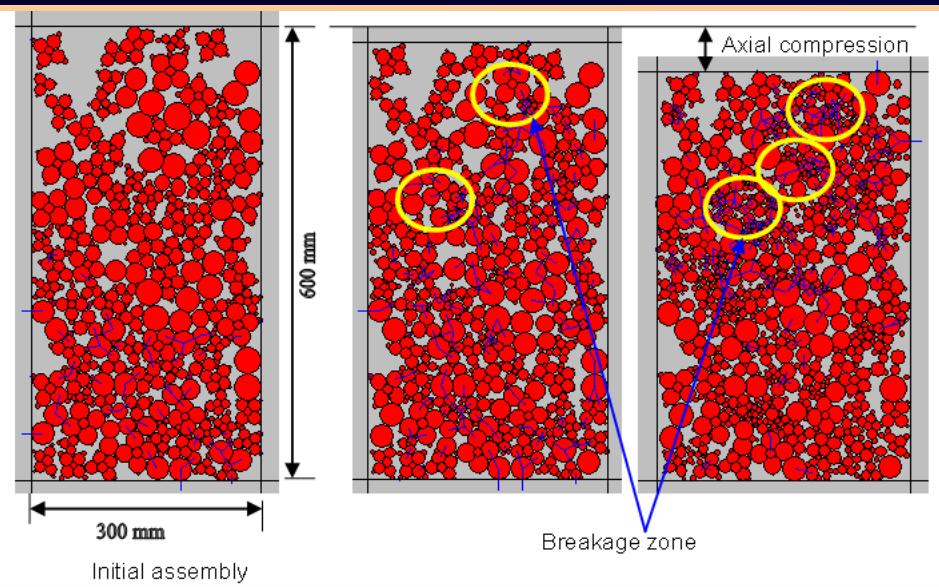
# DEM Modelling of Railway Ballast under Monotonic and Cyclic Triaxial Loading



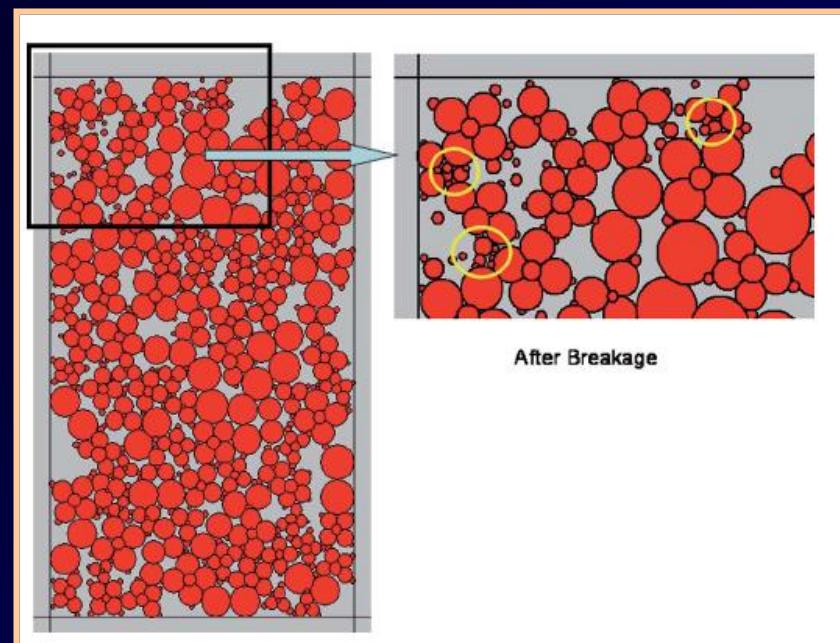
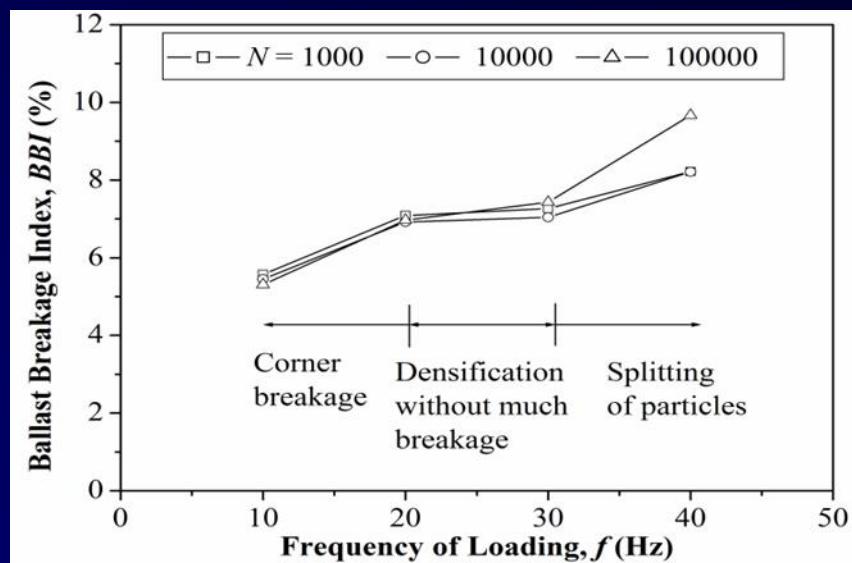
McDowell, Harireche, Konietzky, Brown, and Thom (2006),  
Proc. ICE-Geotechnical Engineering, 159(1): 35-48.

Lu & McDowell (2010), Géotechnique,  
60(6): 459–467.

# Practical Implications: Train Speed vs Particle Breakage

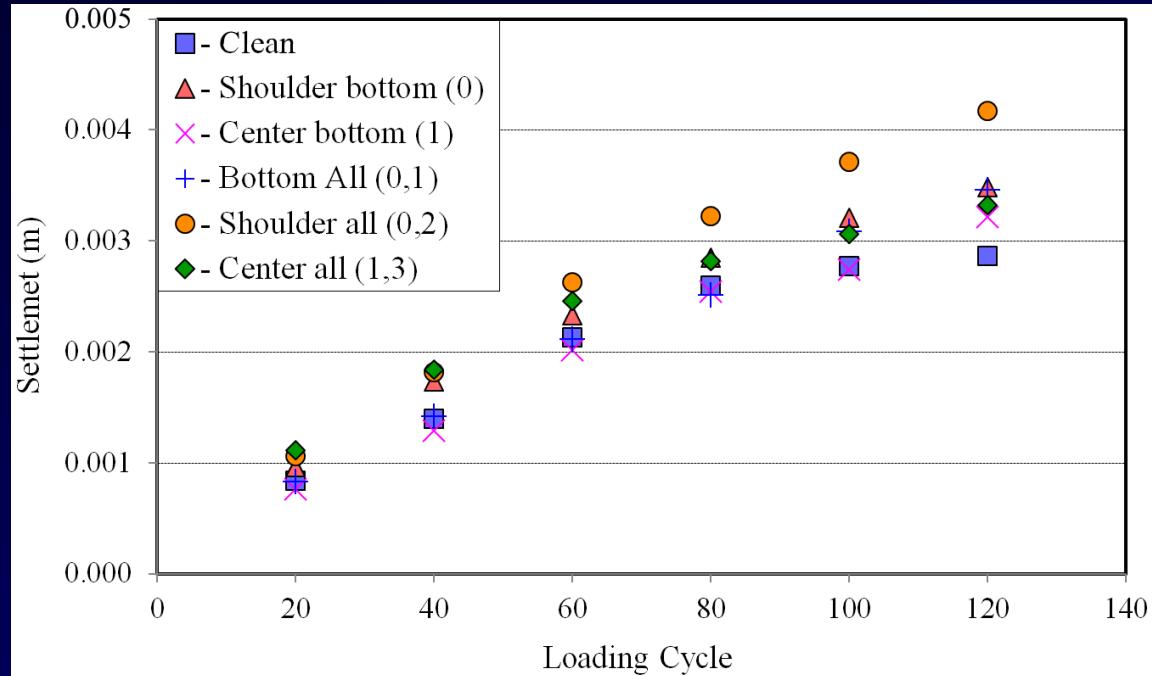
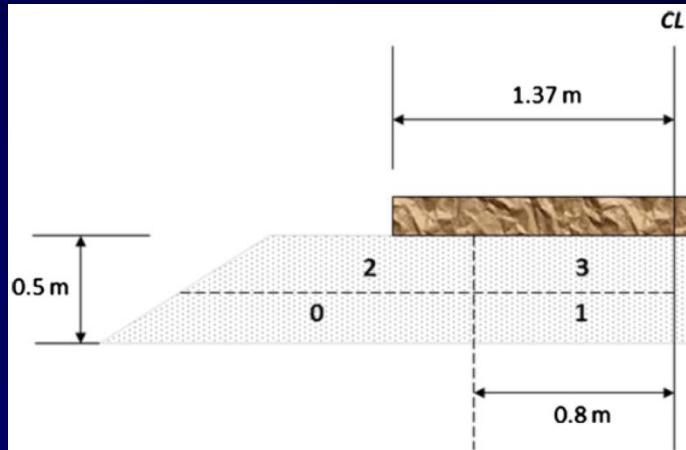
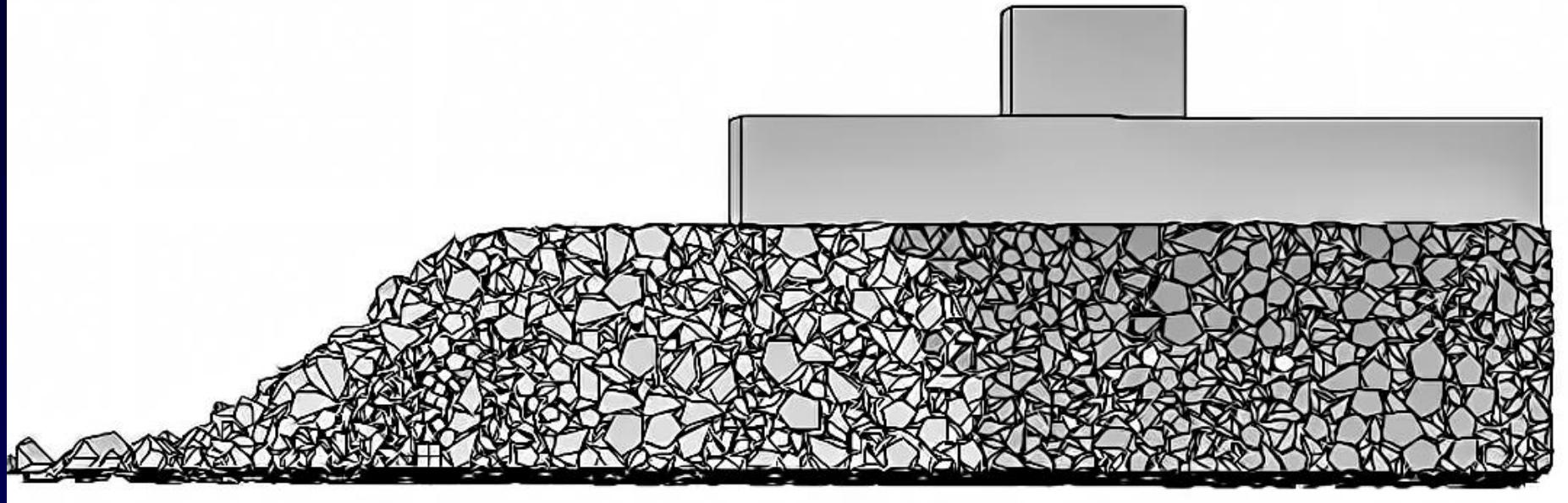


Model particle shapes and sizes



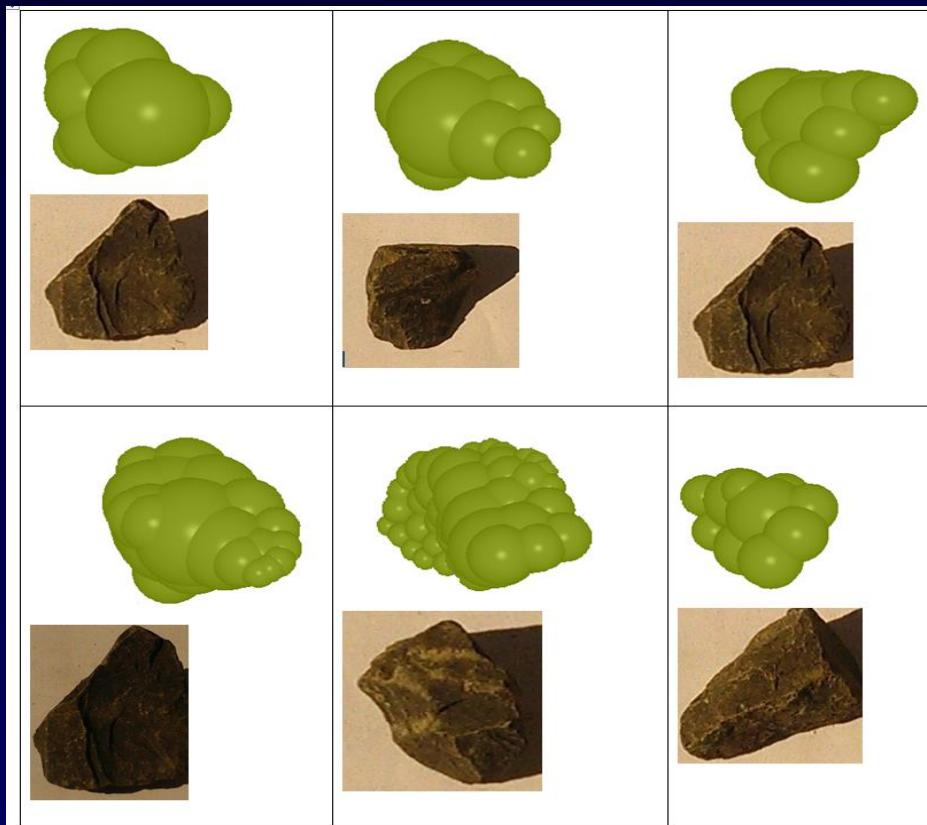
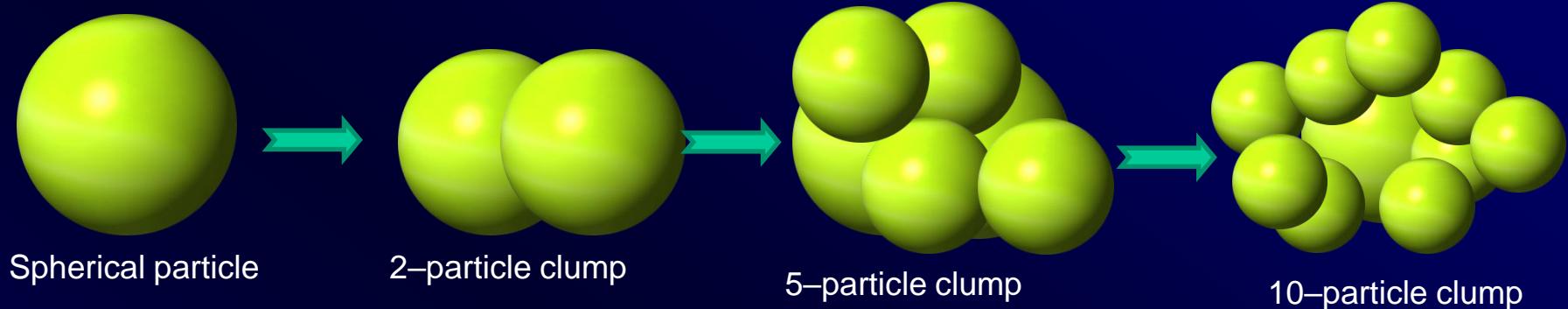
Particle Breakage near the top plate

# DEM Model with Different Parts of Track Fouled by Coal Dust

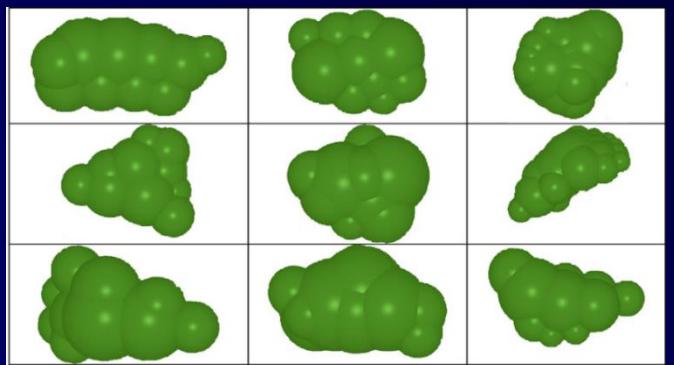
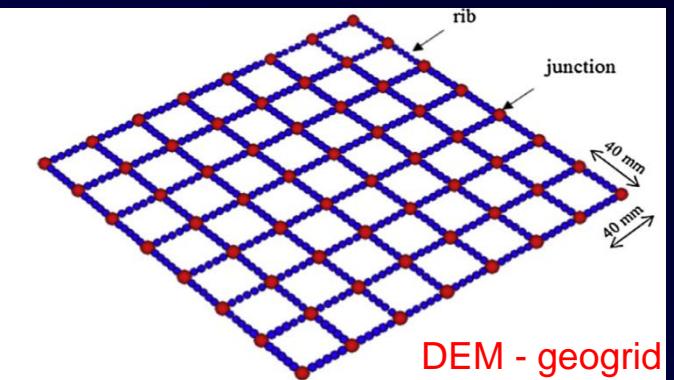
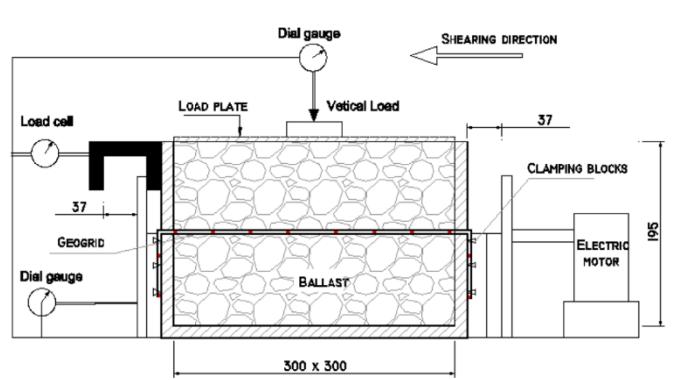


Huang and Tutumluer (2011),  
*Construction and Building Materials*,  
25(8): 3306-3312.

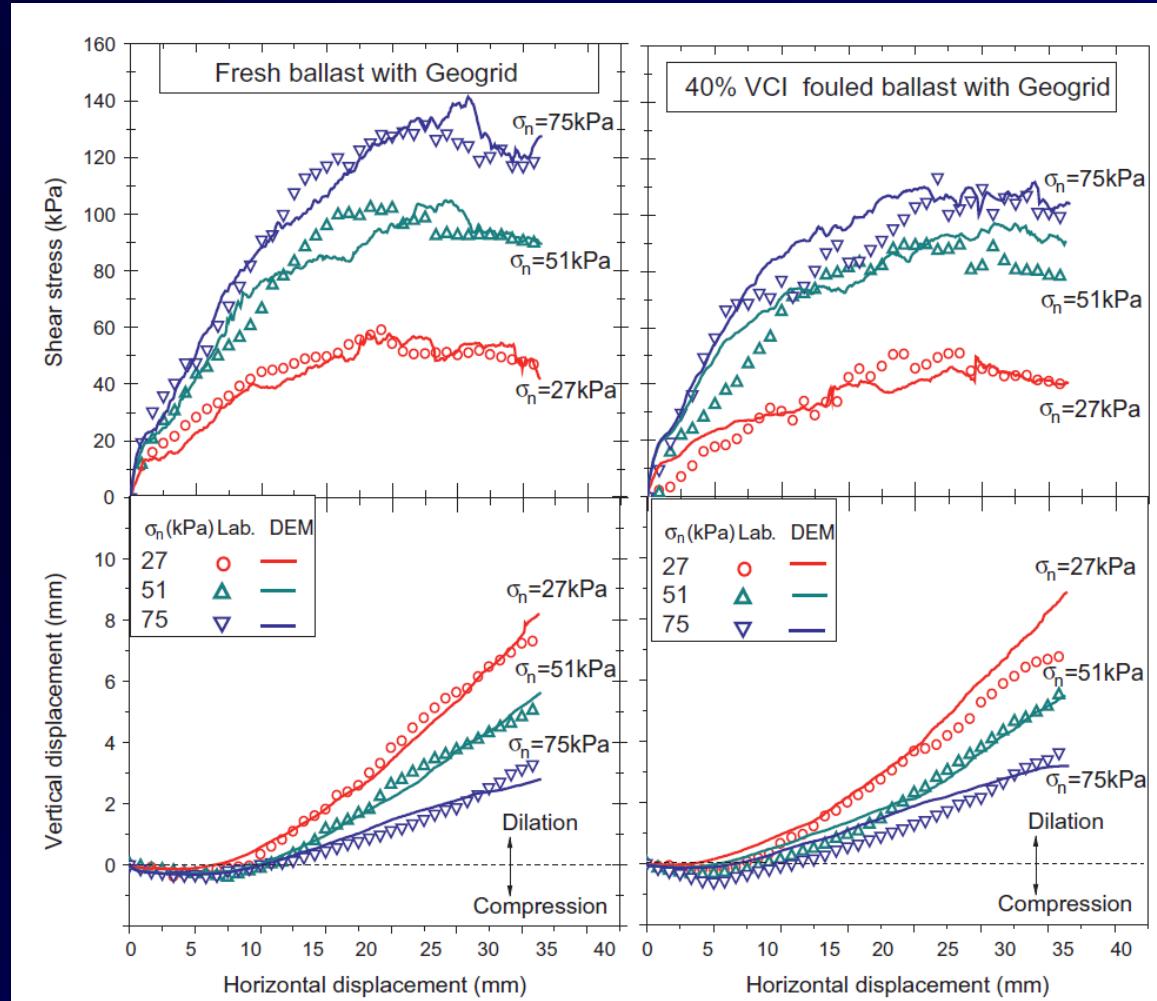
# Modelling particle angularity in DEM



# DEM Modelling Geogrid-reinforced Ballast under Shearing Loads



DEM particle shapes and sizes



Comparison of shear stress and displacements for DEM simulation of reinforced ballast

# DEM Model for Geogrid-reinforced Ballast under Direct Shearing

PFC3D 4.00

Settings: ModelPerspective  
Step 16300 14:02:03 Fri Sep 09 2011

Center: Rotation  
X: 2.046e-001 X: 25.000  
Y: 1.148e-001 Y: 0.000  
Z: 5.330e-002 Z: 120.000  
Dist: 2.059e+000 Mag.: 1  
Ang.: 22.500

## Group

- layer5
- layer2
- layer1
- layer4
- layer7
- layer6
- layer3
- rpl-000076

## Axes

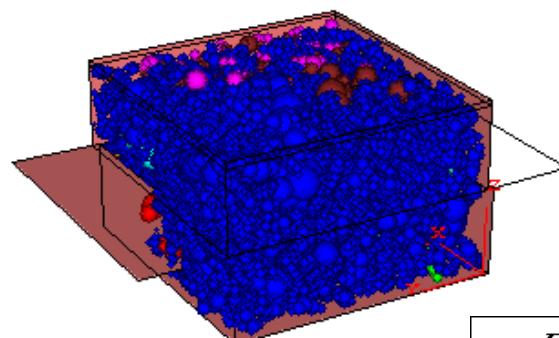
Linestyle

Wall

Cluster

Ball

View Title: Direct Shear Testing of Fresh Ballast



PFC3D 4.00

Step 16300 12:02:35 Fri Sep 09 2011

## Table

51 UnNamed  
Linestyle  
0.000e+000 <> 1.029e+005

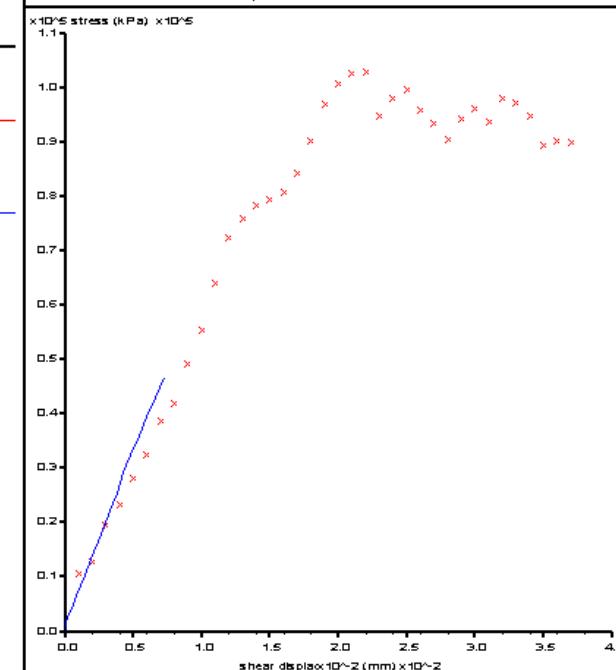
## History

1 shearstress (FISH Symbol)  
Linestyle  
6.489e+002 <> 4.675e+004

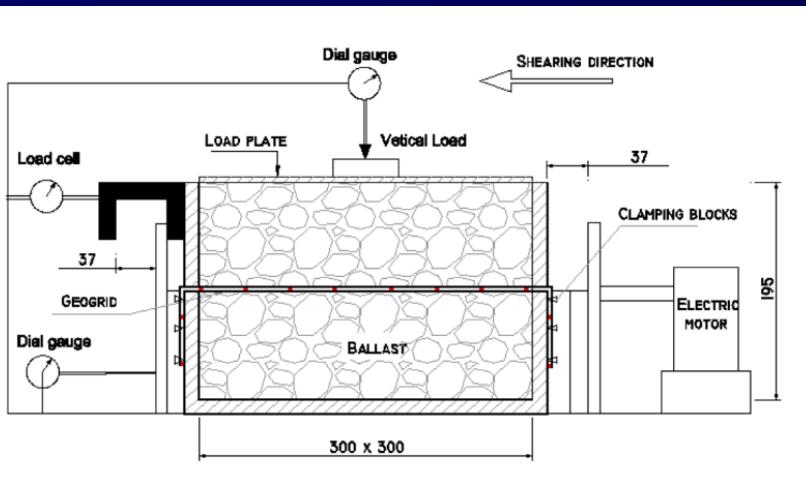
## Vs.

2 ydisp2 (FISH Symbol)  
1.679e-005 <> 7.284e-003

View Title: shear stress vs shear displacement

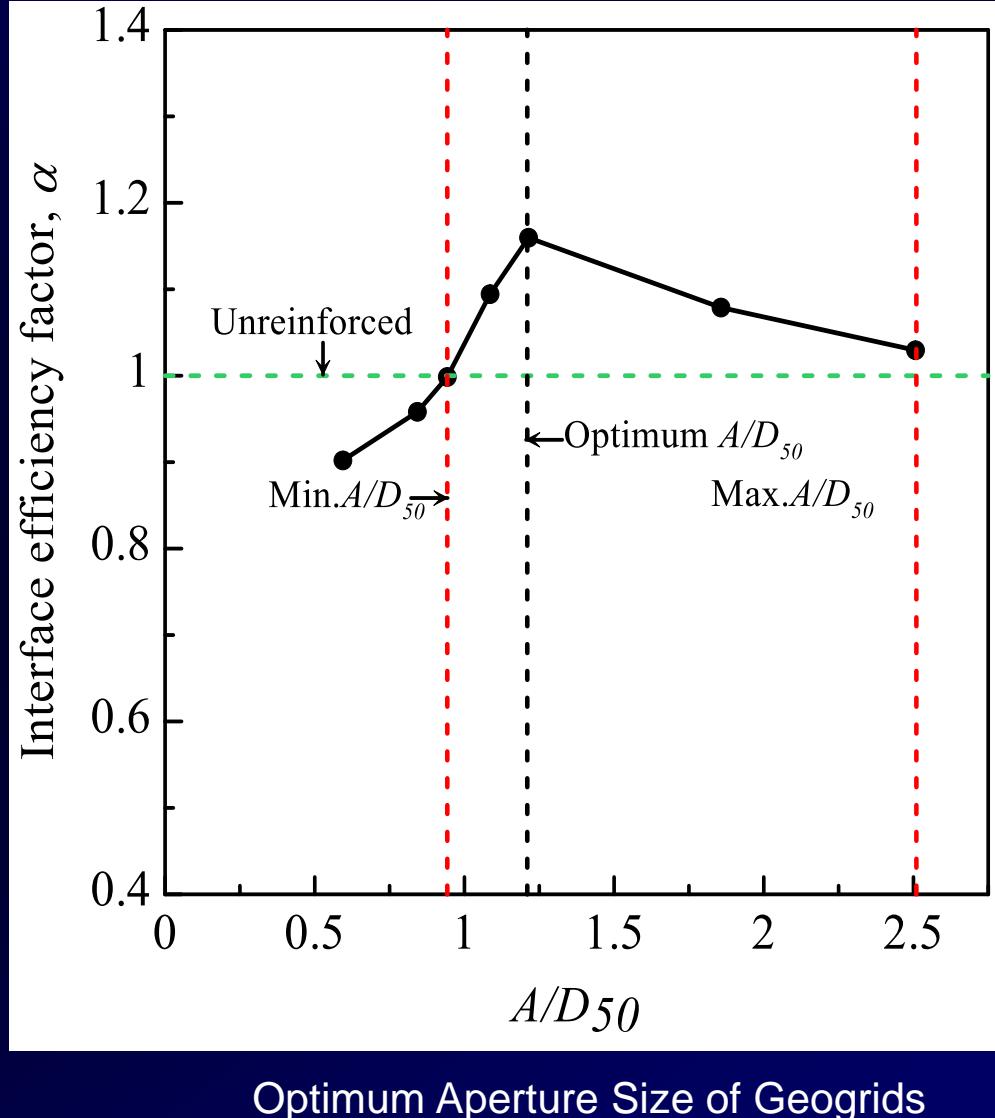


# Geogrids for preventing particle movement and breakage

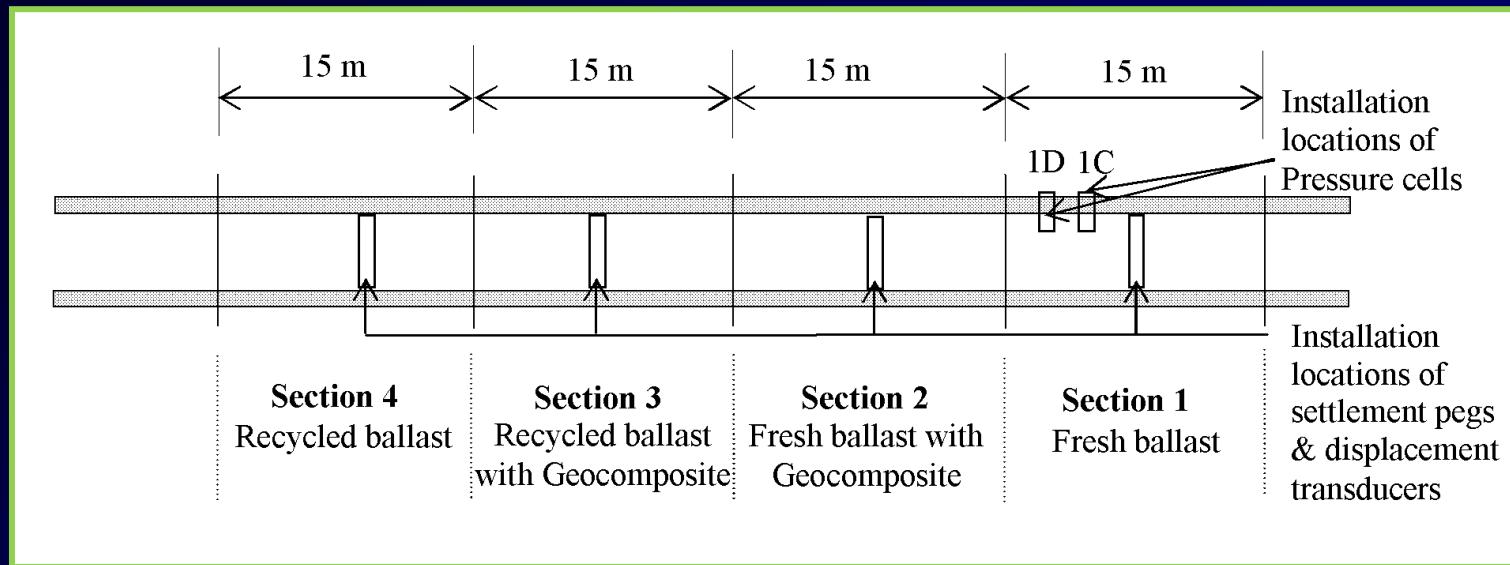


## Geogrids Used for Testing

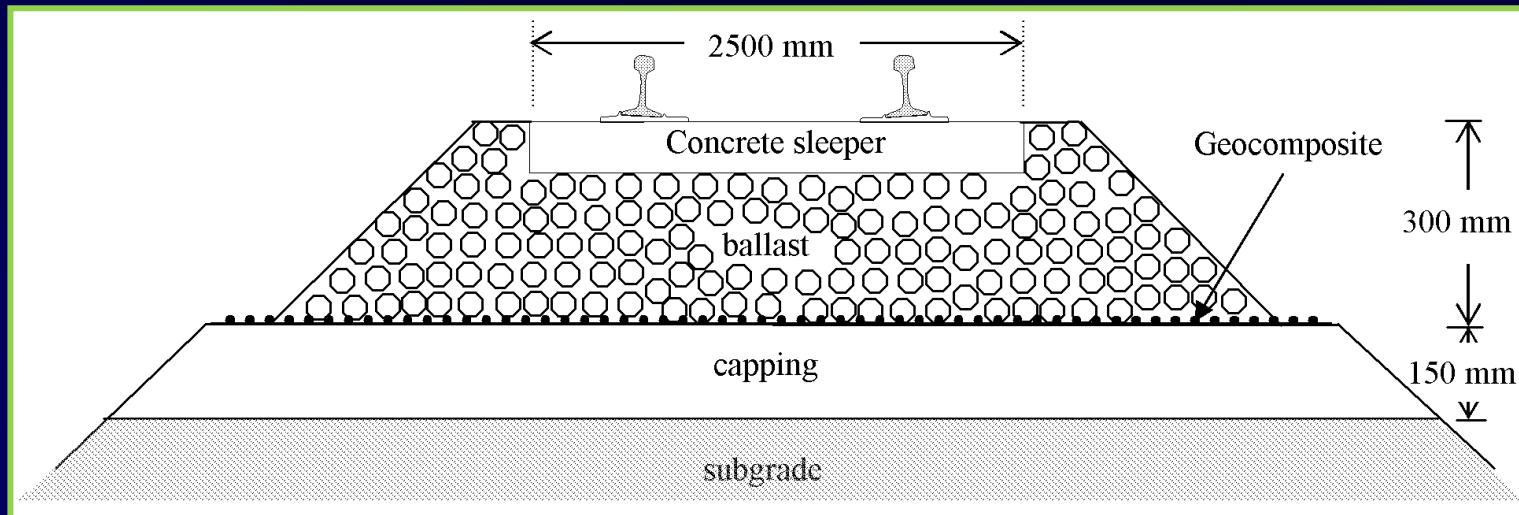
Geogrid type	Aperture shape	Aperture size (mm)	$T_{ult}$ (kN/m)
G1	Square	38 x 38	30
G2	Triangle	36	19
G3	Square	65 x 65	30
G4	Rectangle	44 x 42	30
G5	Rectangle	36 x 24	30
G6	Square	33 x 33	40
G7	Rectangle	70 x 110	20



# Field Trial on Instrumented Track near Wollongong (Bulli)



## Details of instrumented track



Section of ballasted track bed with geocomposite layer

# Field Trial on Instrumented Track – Town of Bulli



Geocomposite layer (geogrid+geotextile)  
before ballast placement



Ballast placement over  
the geocomposite

8 October 2006



Recycled Ballast  
from Chullora Quarry, Sydney

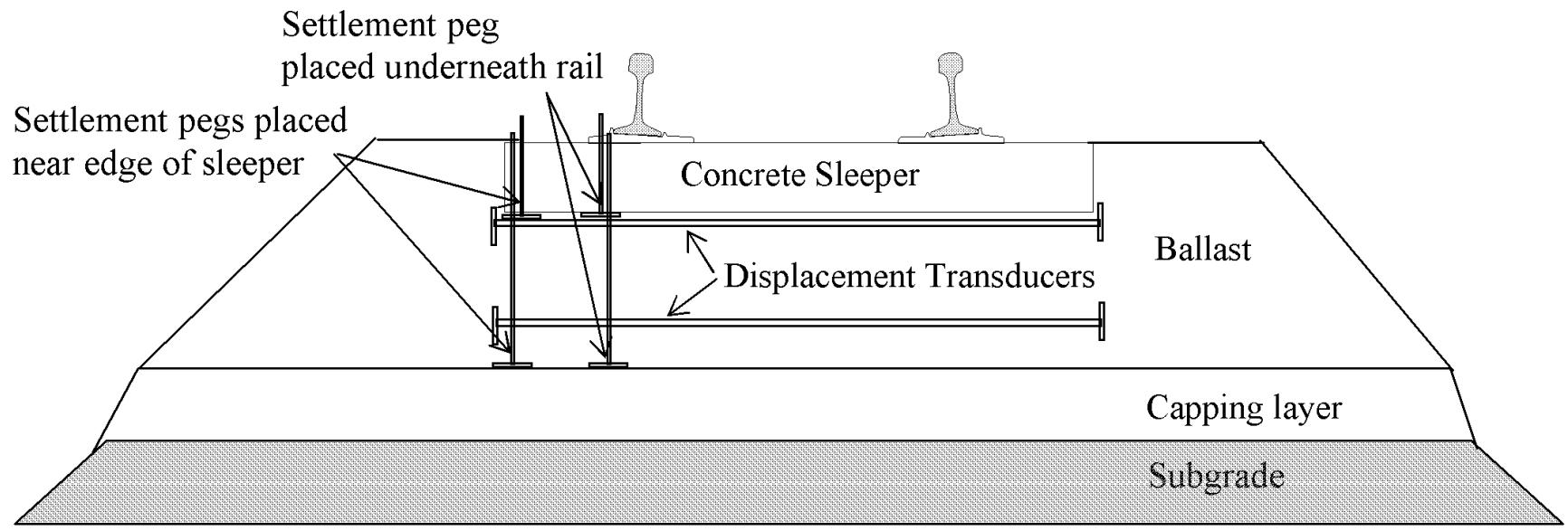


Fresh Ballast  
Bombo Quarry, Wollongong

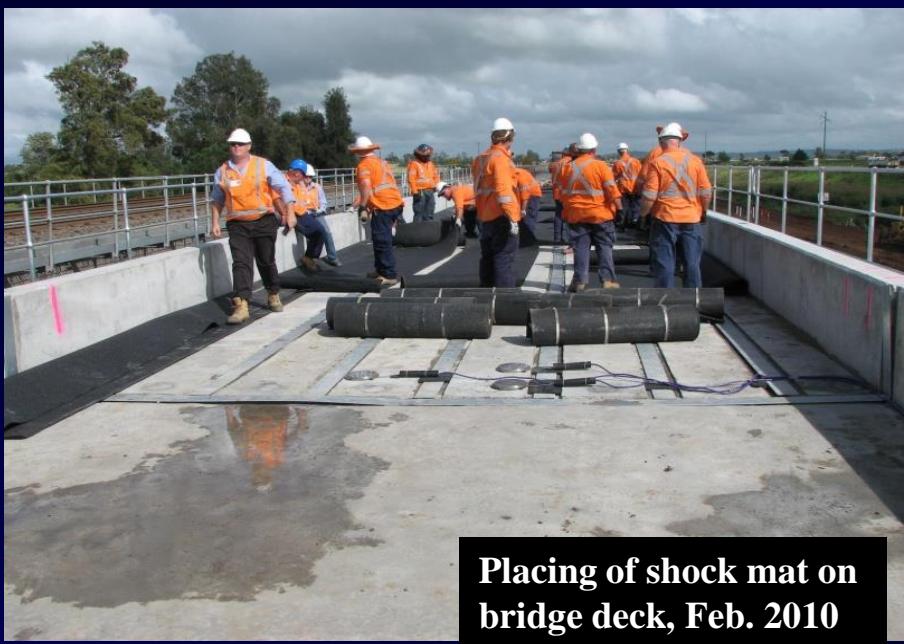


Bonded Geogrid

# Field Instrumentation – Town of Bulli



# Field Monitoring: Town of Singleton near Newcastle (NSW)

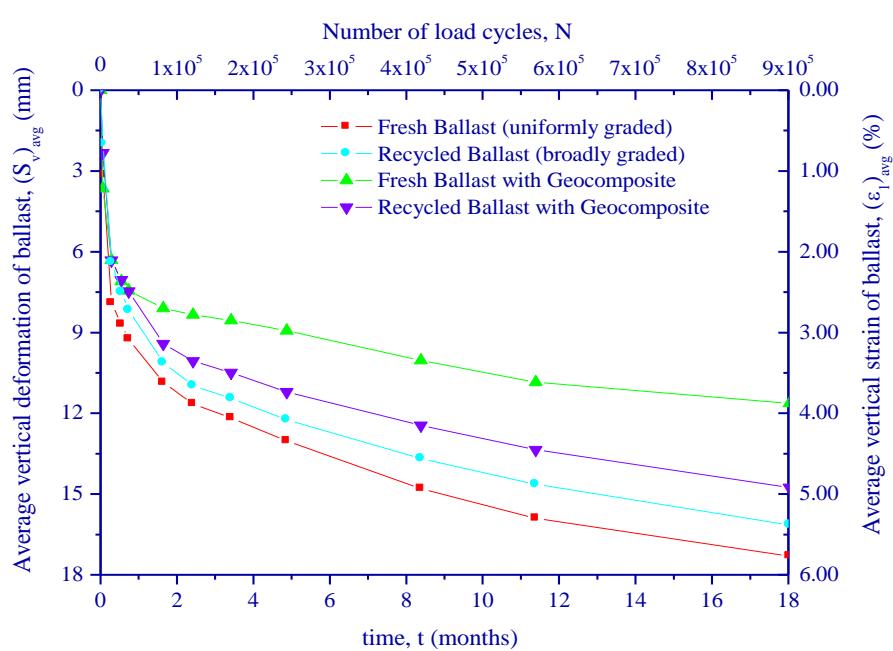


# Role of Geosynthetics - Field Monitoring of Track Response

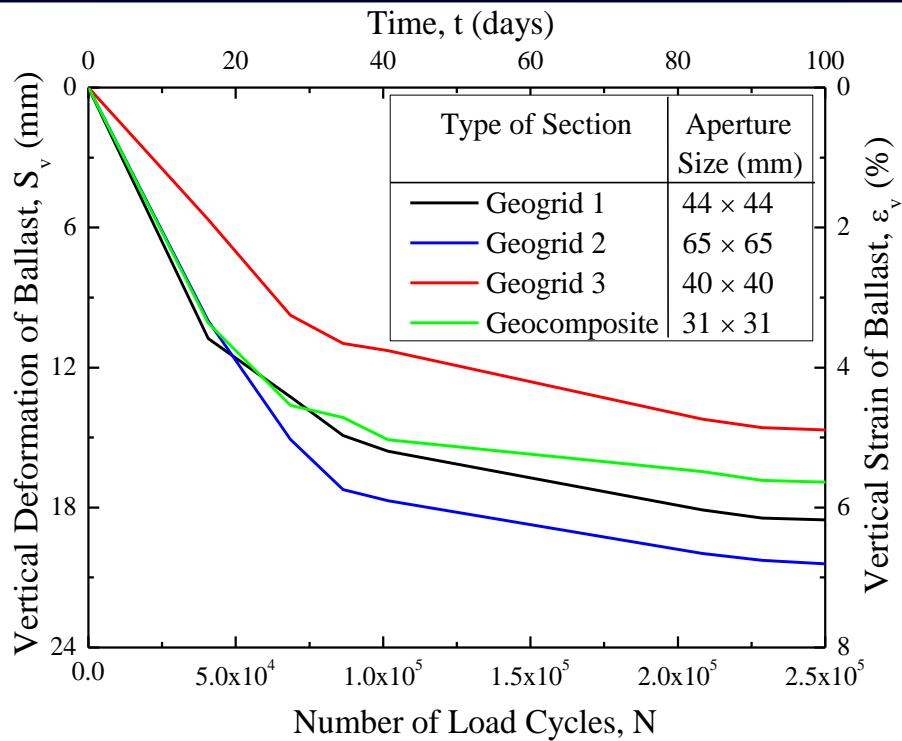
Bulli Track: Indraratna et al. (2010). JGGE, ASCE, Vol. 136(7), 907-917

Singleton Track: Indraratna et al. (2014). ICE Proc. Ground Improvement, 167(1), 24-34

## Bulli Track



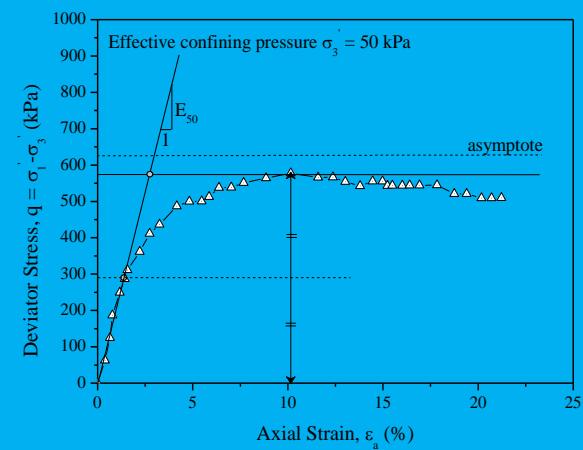
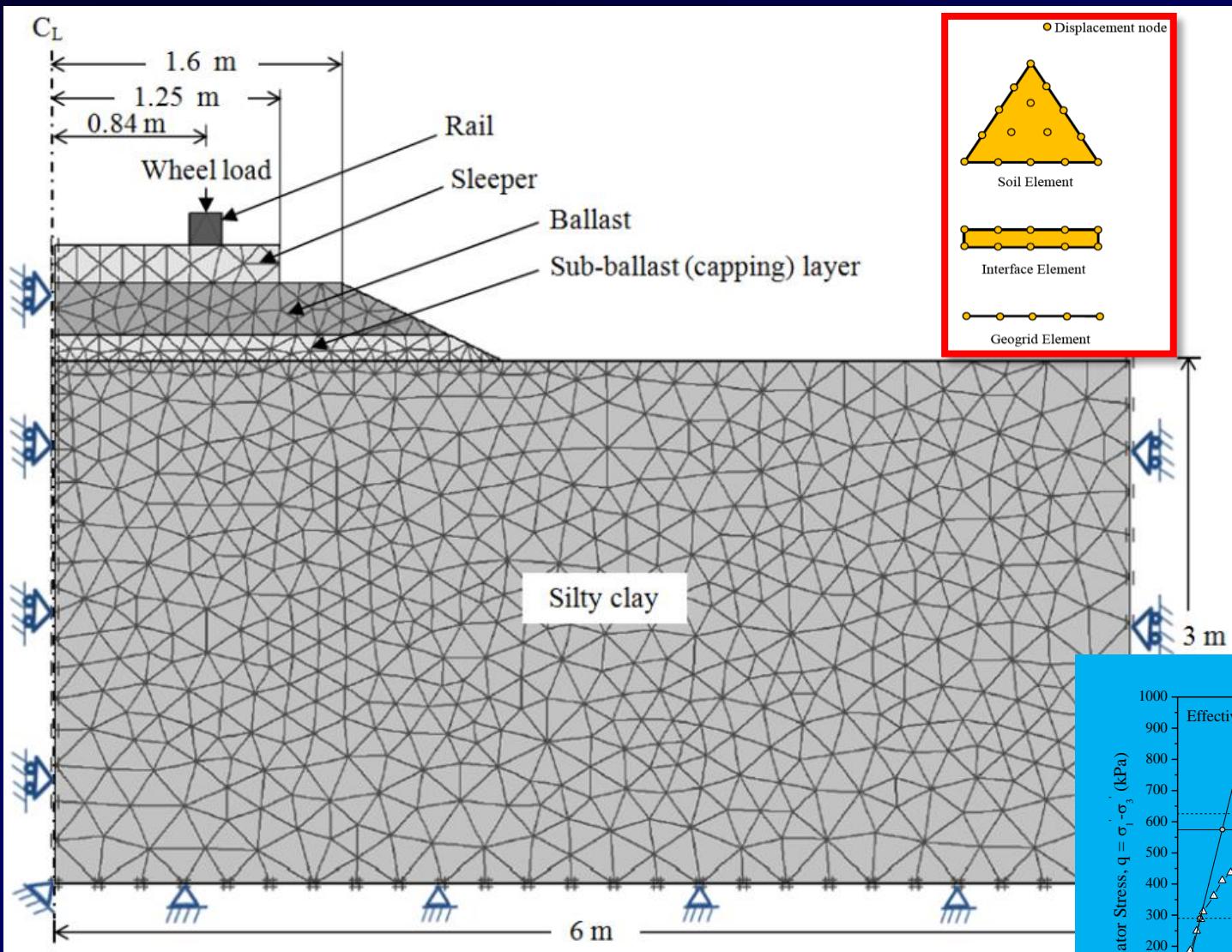
## Singleton Track



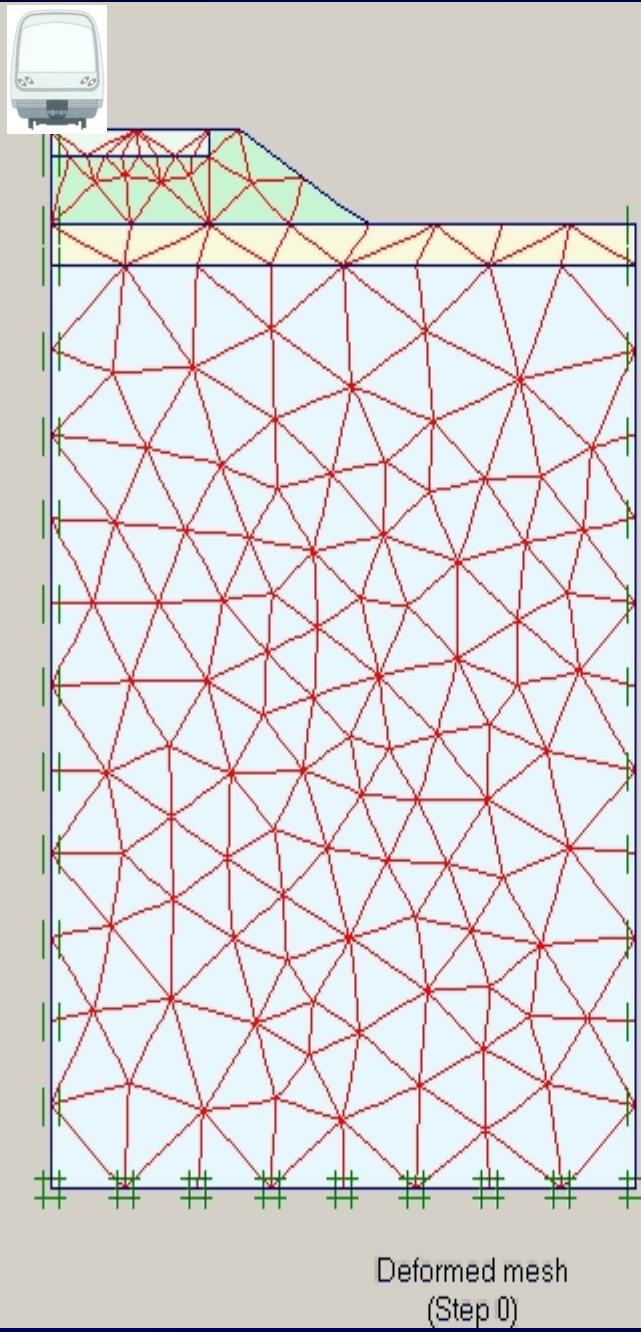
Recycled ballast: broadly-graded compared to uniform fresh ballast – so performed better !

Optimum aperture size of geogrids is about  $1.2D_{50}$  of ballast.

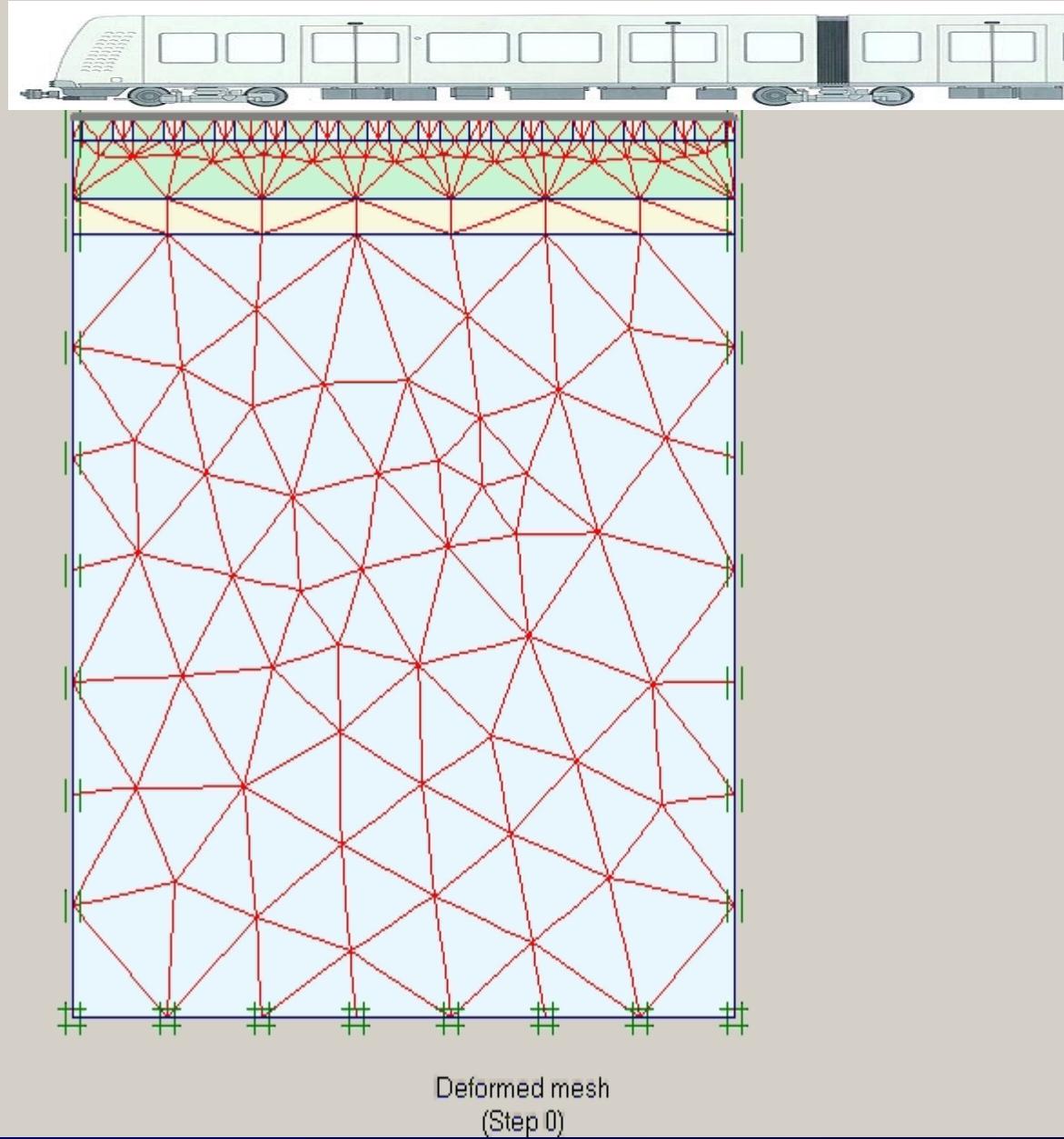
# Finite Element Analysis of Track: 2D Plane Strain



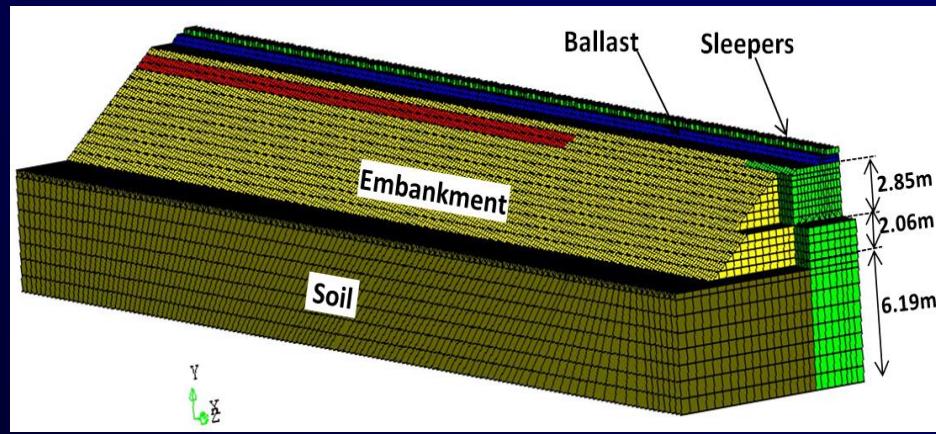
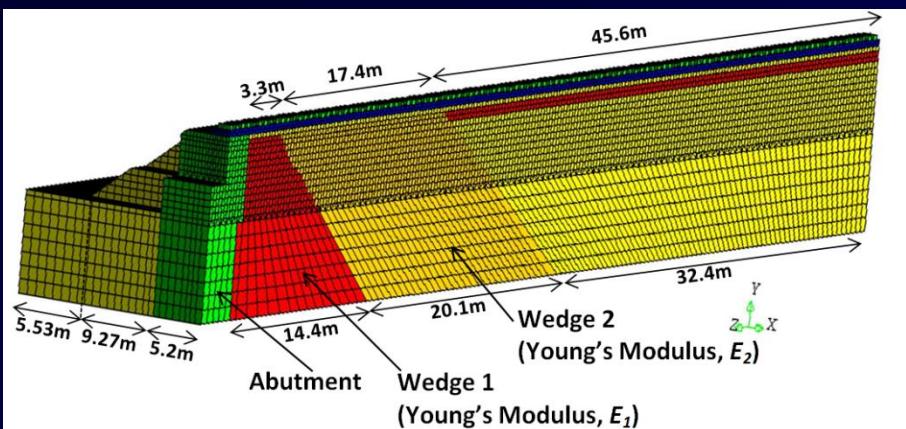
# Track transverse section deformation



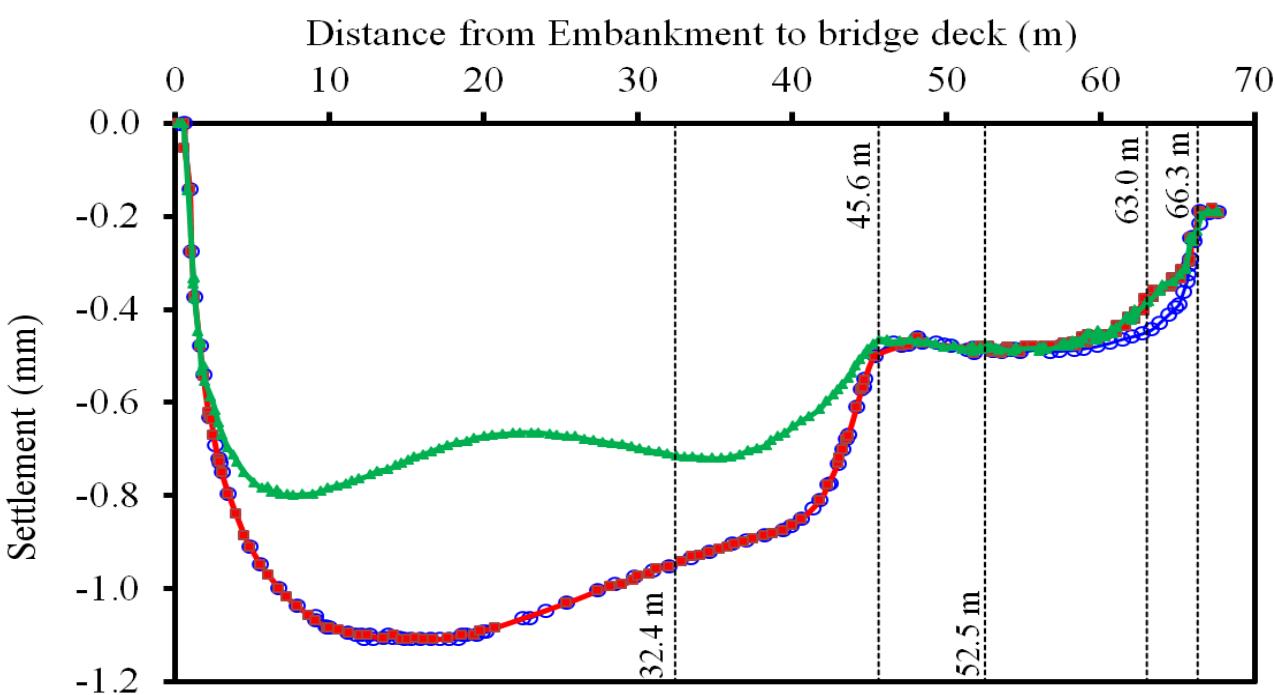
# Track longitudinal section deformation



# FEM modelling of Transition Zones



- Model 1:  $E_{\text{Embankment}} = 60 \text{ MPa}$ ,  $E_1/E_2 = 1.195$
- Model 2:  $E_{\text{Embankment}} = 60 \text{ MPa}$ ,  $E_1/E_2 = 2.0$
- Model 3:  $E_{\text{Embankment}} = 100 \text{ MPa}$ ,  $E_1/E_2 = 2.0$

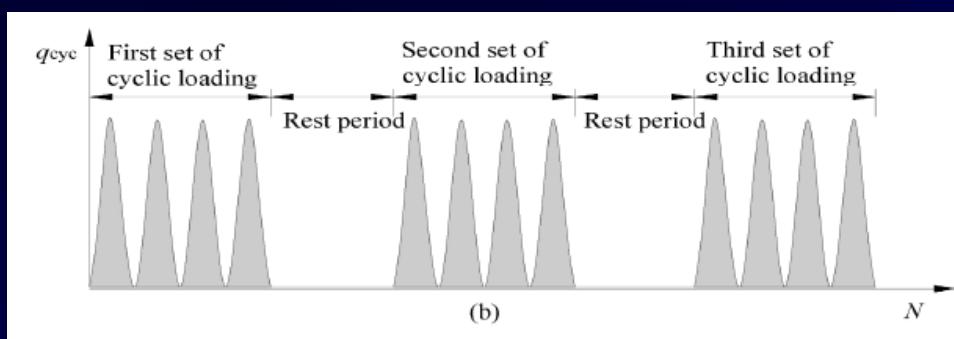
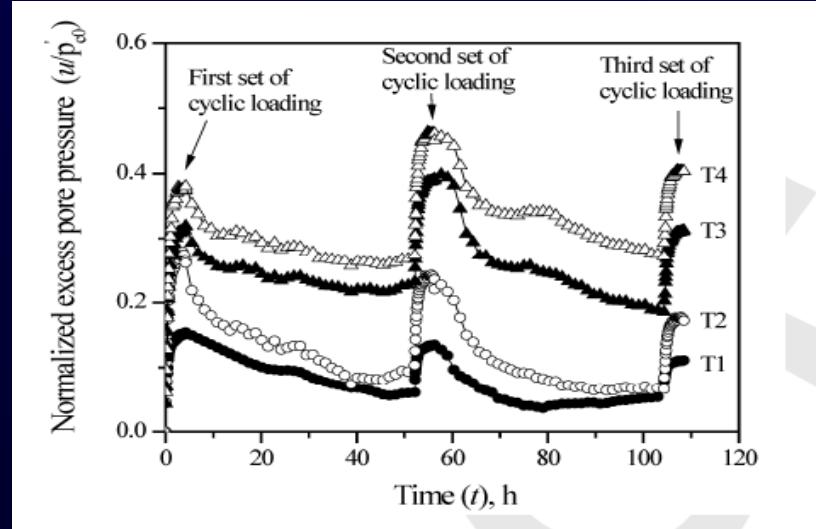
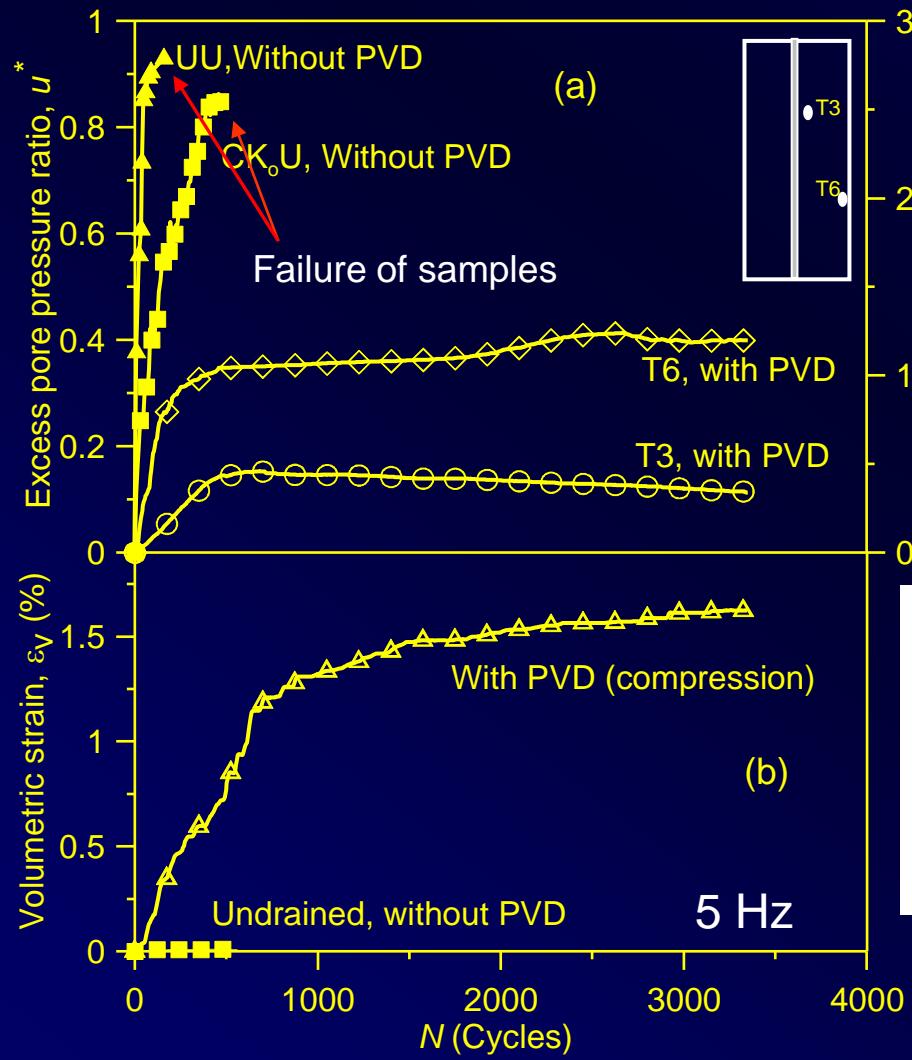


Performance assessment  
and numerical solutions  
for transition zones –  
approaching bridge deck

Seara and Correia (2010), Semana de Engenharia Escola de Engenharia da Universidade do Minho.

# Cyclic Response of Soft Subgrade with Vertical Drains under High Speed Rail Conditions

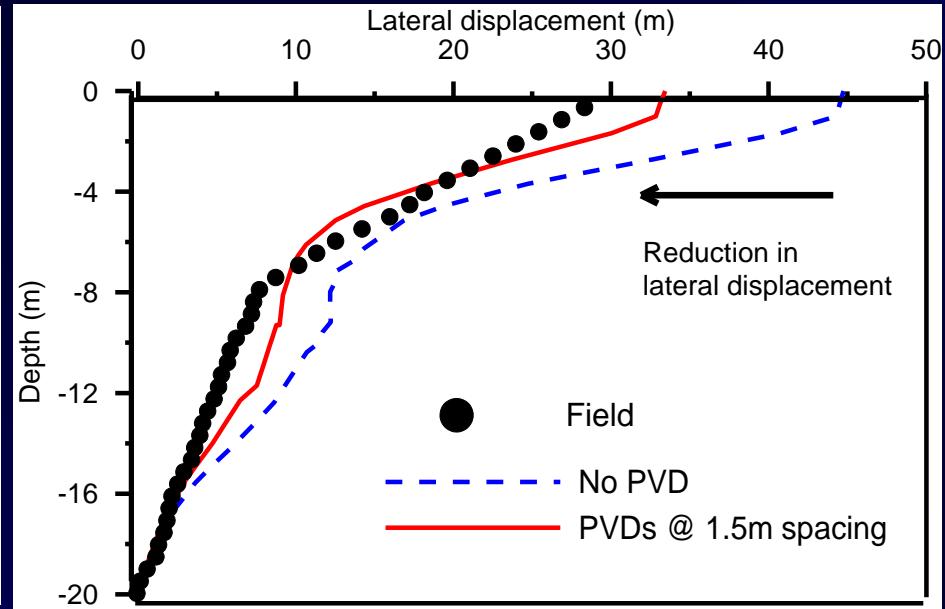
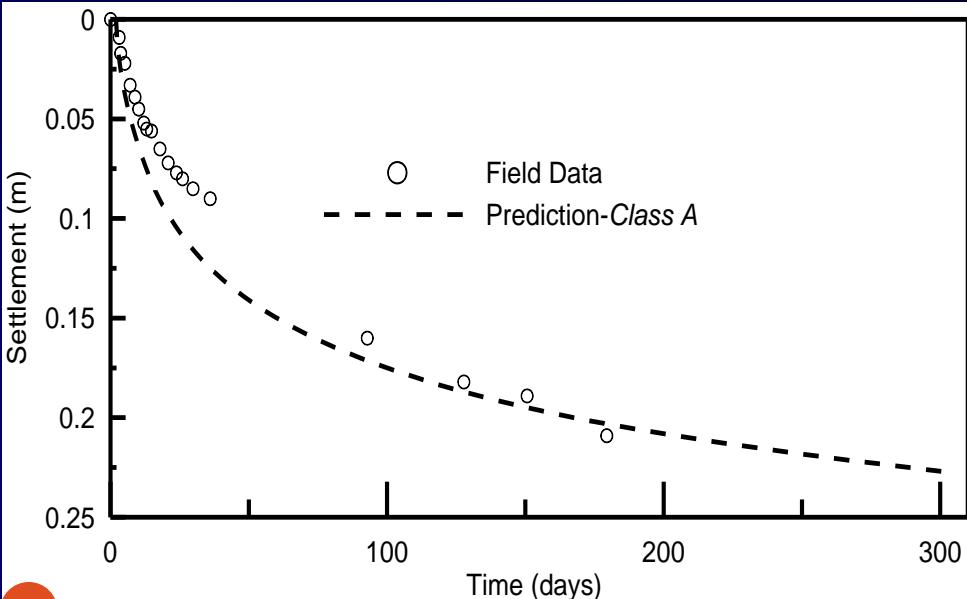
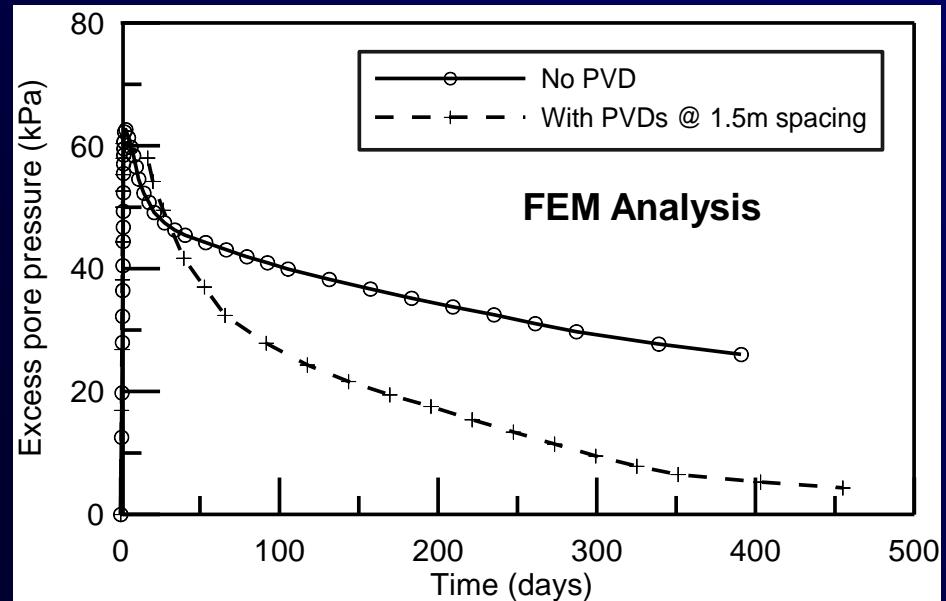
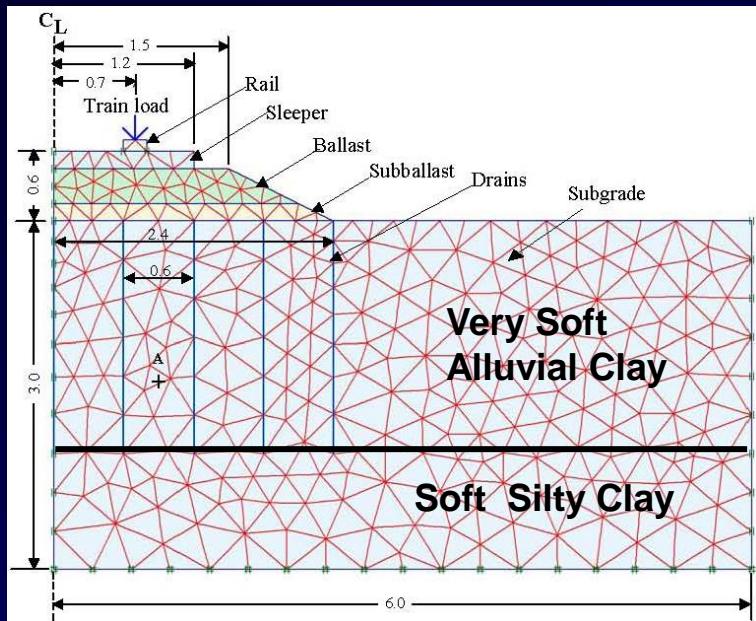
Indraratna, Attya and Rujikiatkamjorn (2009) JGGE, ASCE, Vol. 135(6), 835-839



Specimens without PVD fail very quickly as the excess pore pressure rises rapidly!

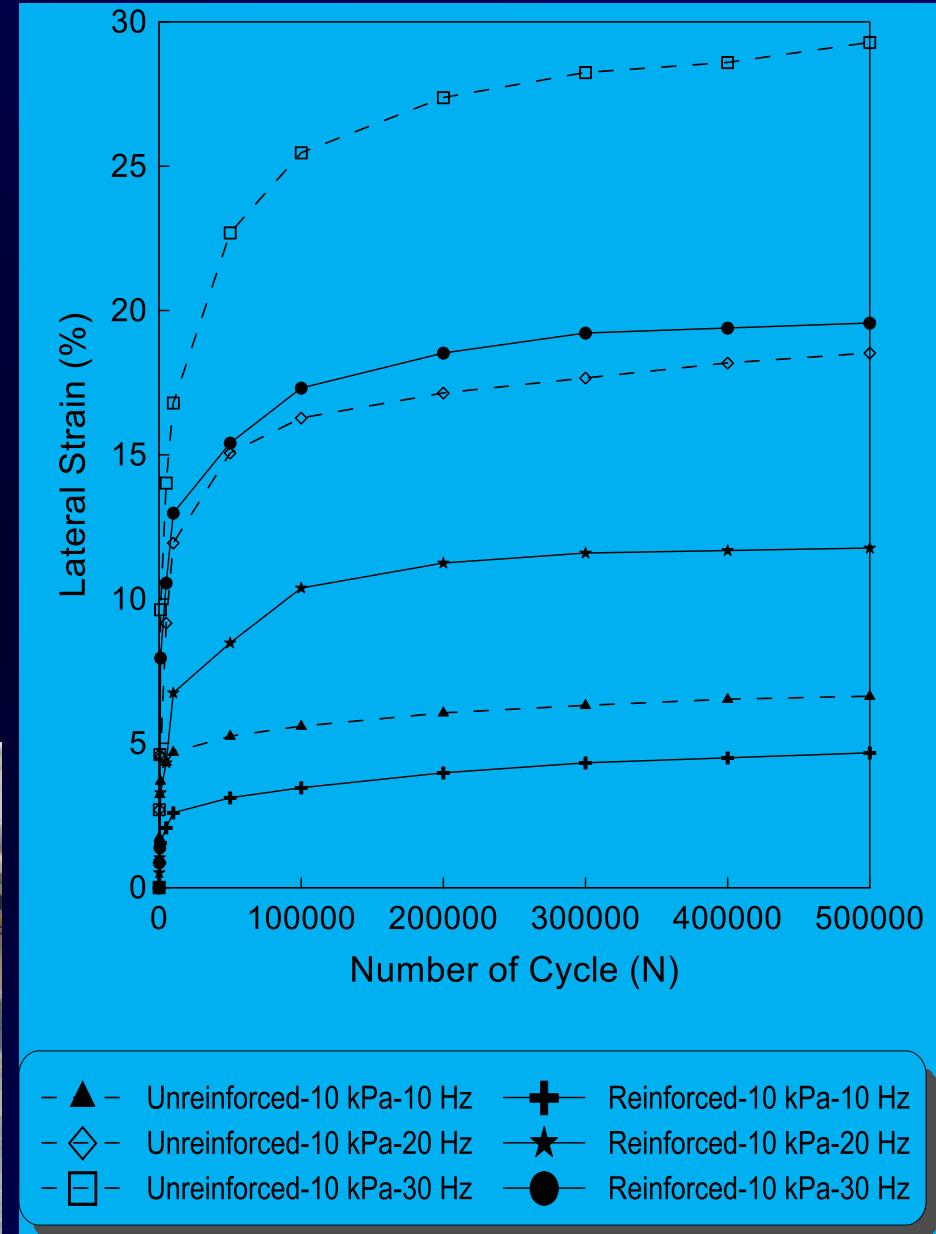
# Short PVD Applications to Rail Embankment at Sandgate and FEM Analysis

## Class A Prediction (Indraratna et al, ASCE, JGGE, 2010)



# Geocell stabilisation of capping layer to minimise mud pumping

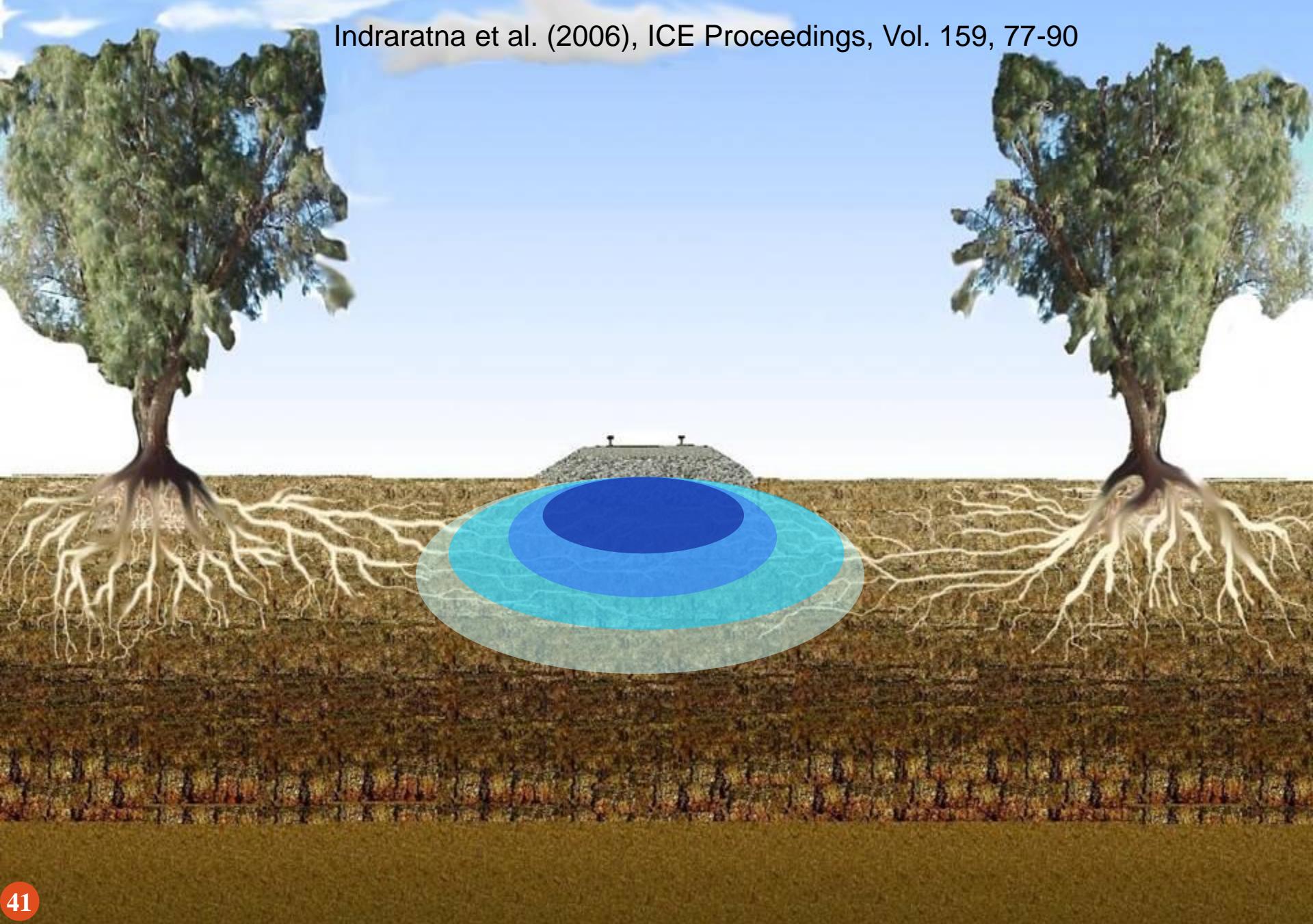
Indraratna et al. (2015). JGGE, ASCE, Vol. 141(1), 1-16



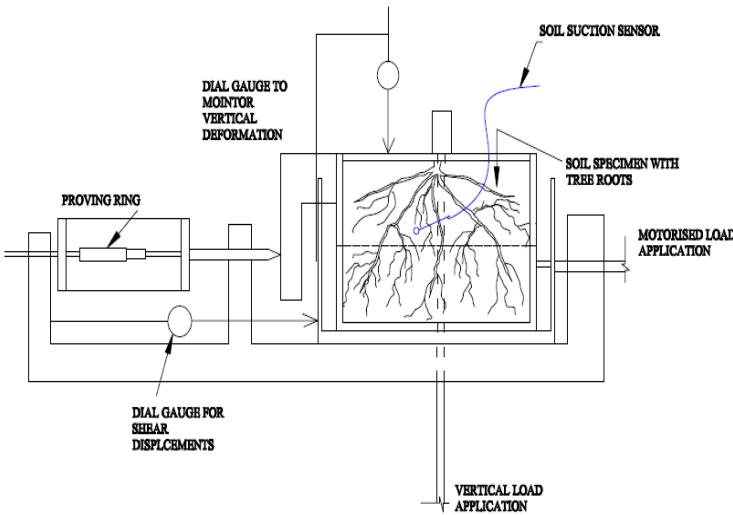
Geocells provide confinement to the capping layer and prevent lateral spreading

# Interaction of Trees and Ground for Stabilising Rail Corridors

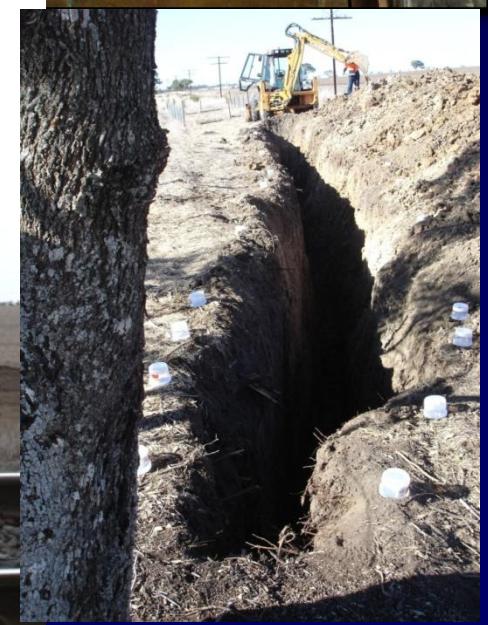
Indraratna et al. (2006), ICE Proceedings, Vol. 159, 77-90



# Laboratory and Field Measurements: Role of Soil Suction

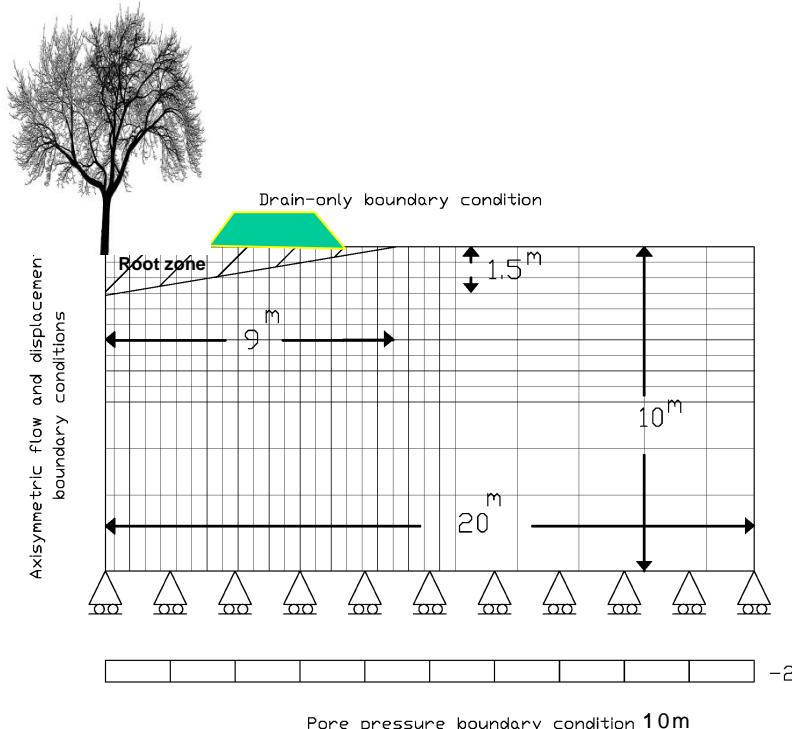


Tensiometer- suction



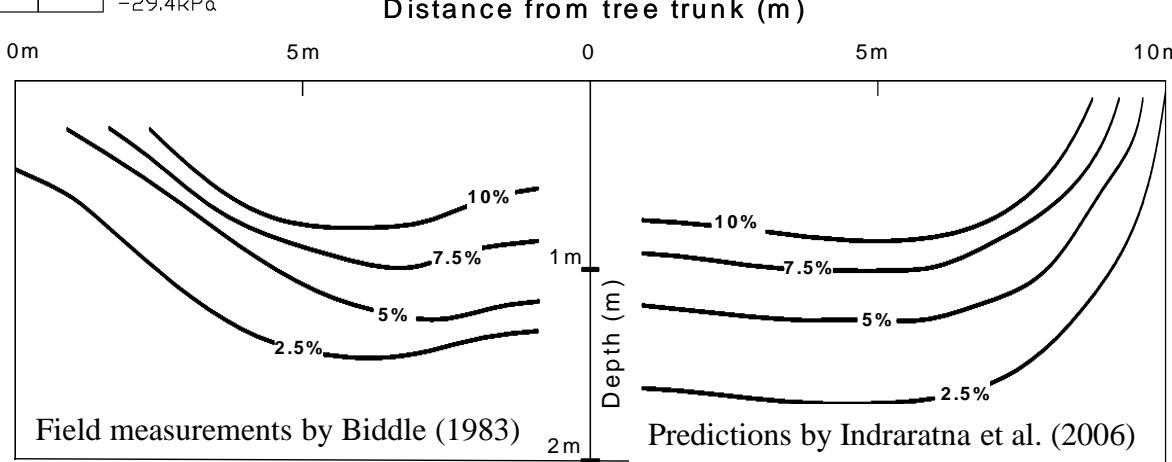
# Computational Tree Root Model Validation

Example: A single, 14m high lime tree in U.K (Biddle 1983)



## FEM modeling of a single native tree

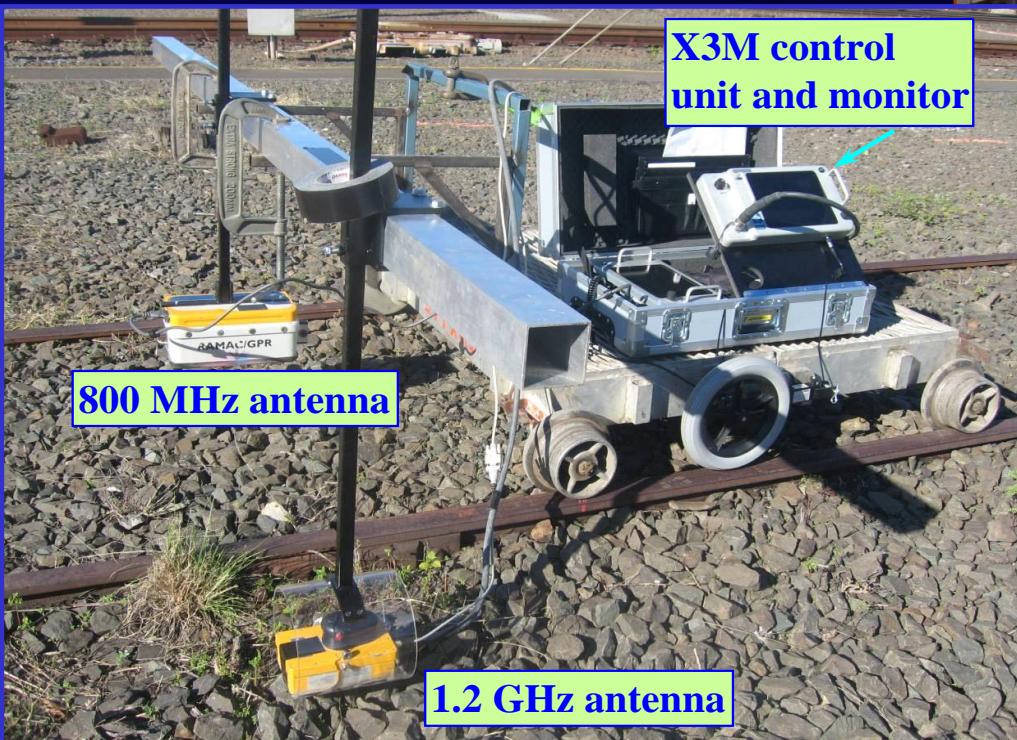
**Contours of volumetric soil moisture content reduction (%) in the vicinity of a lime tree**



# Track Condition Monitoring - Ground Penetration Radar (GPR)

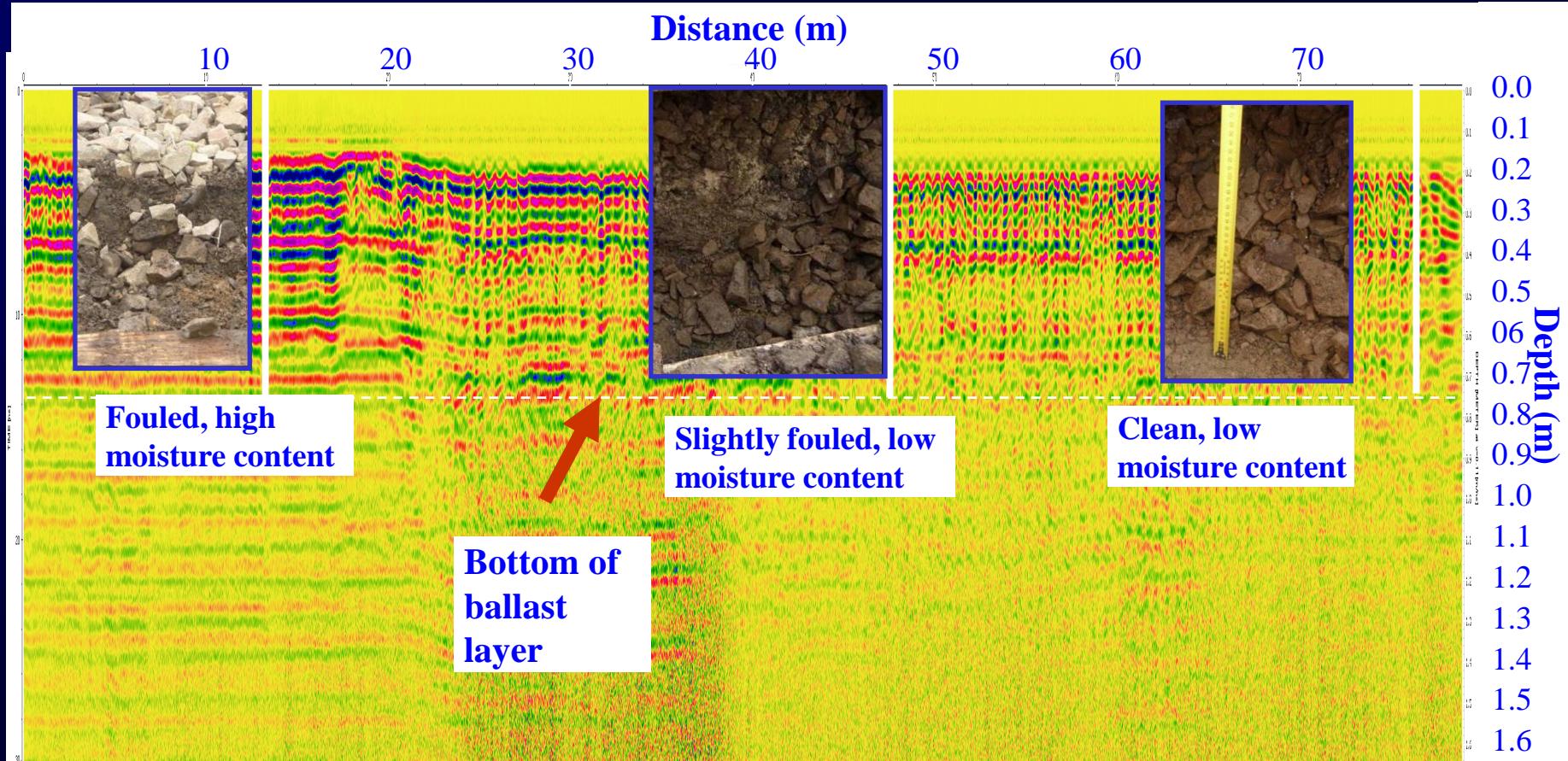


Lab testing (Uni. of Wollongong)



Field testing (Wollongong)

# GPR Lab and Field Testing

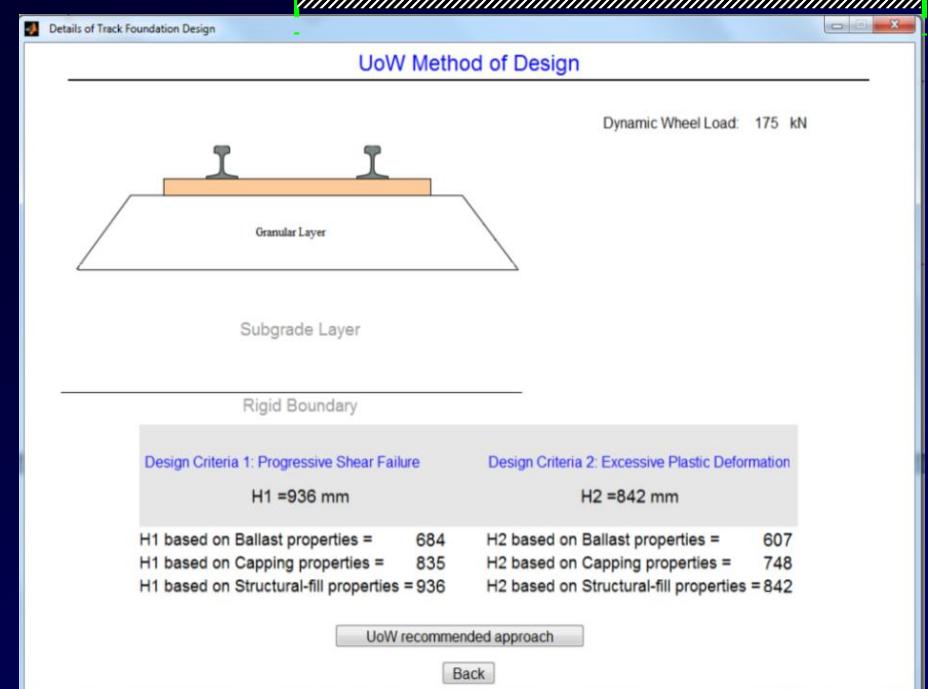
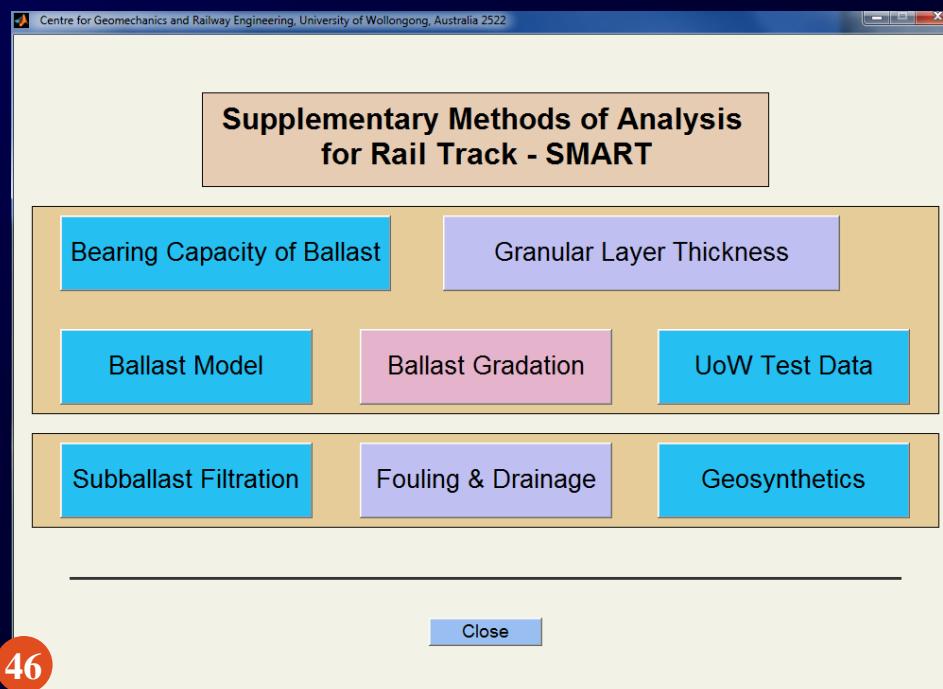
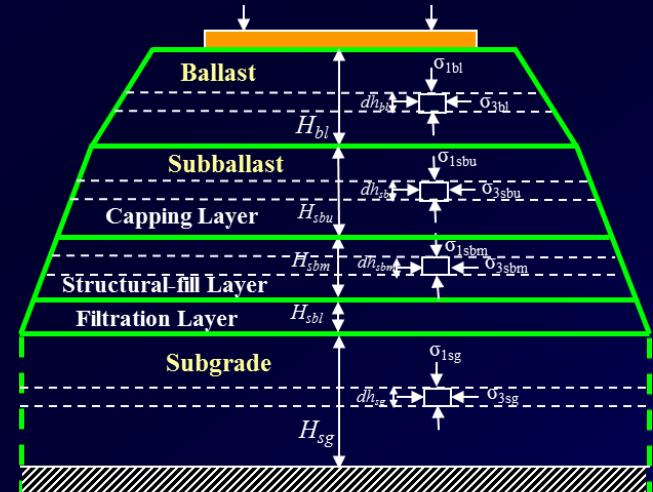


# New Design Procedures – UoW Method

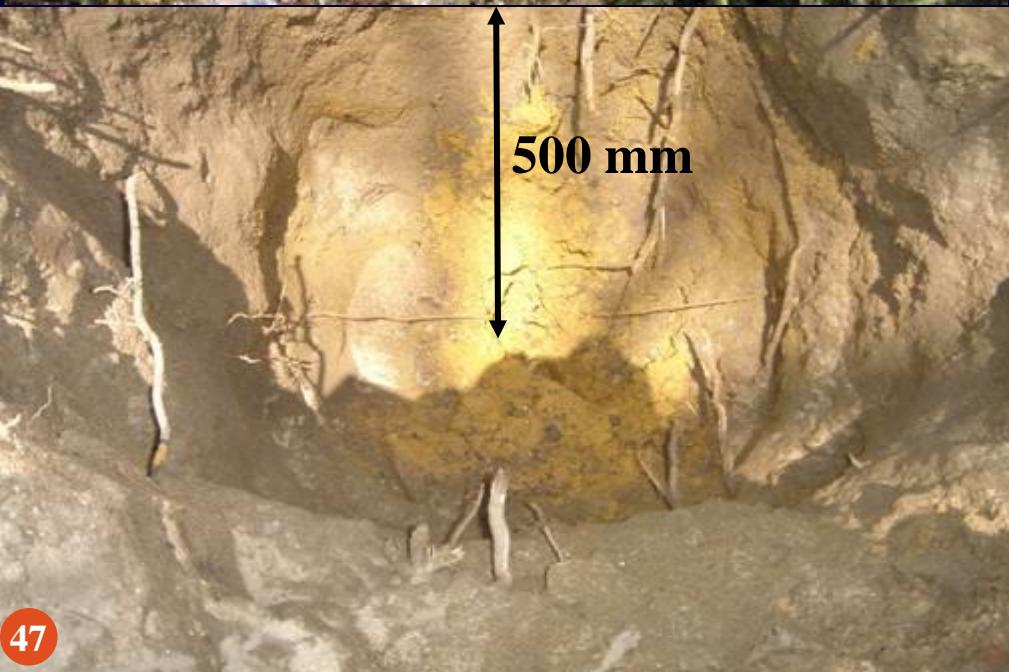
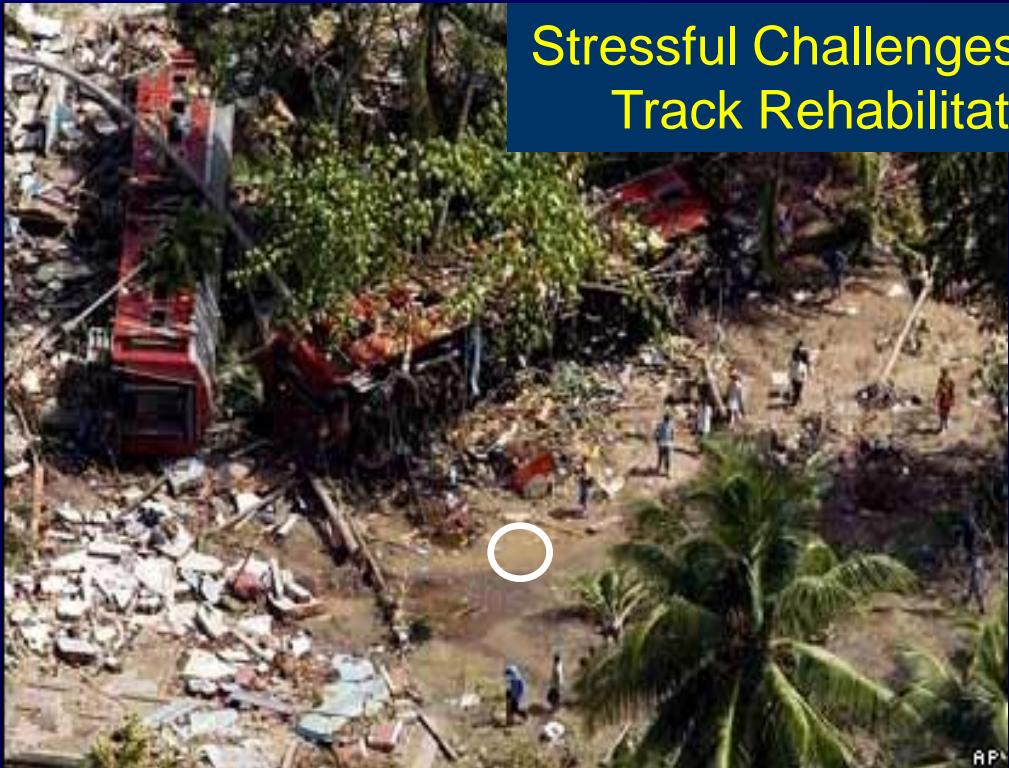
## (Supplementary Method of Analysis of Rail Track – SMART)

Key features:

- a set of MATLAB subroutines: design and analysis of track based on research outcomes;
- a collection of performance-based methods for formation-track analysis.
- stand-alone computer application: user-friendly interfaces for data input and output.

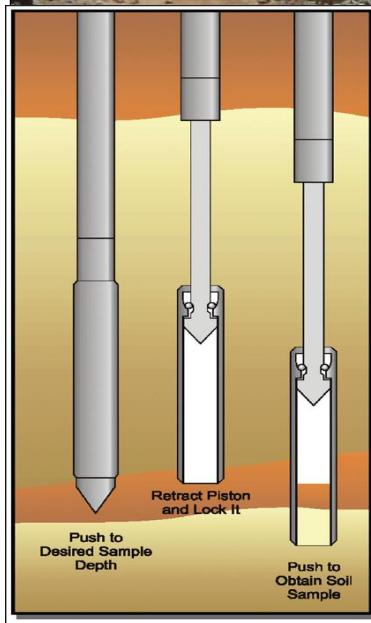
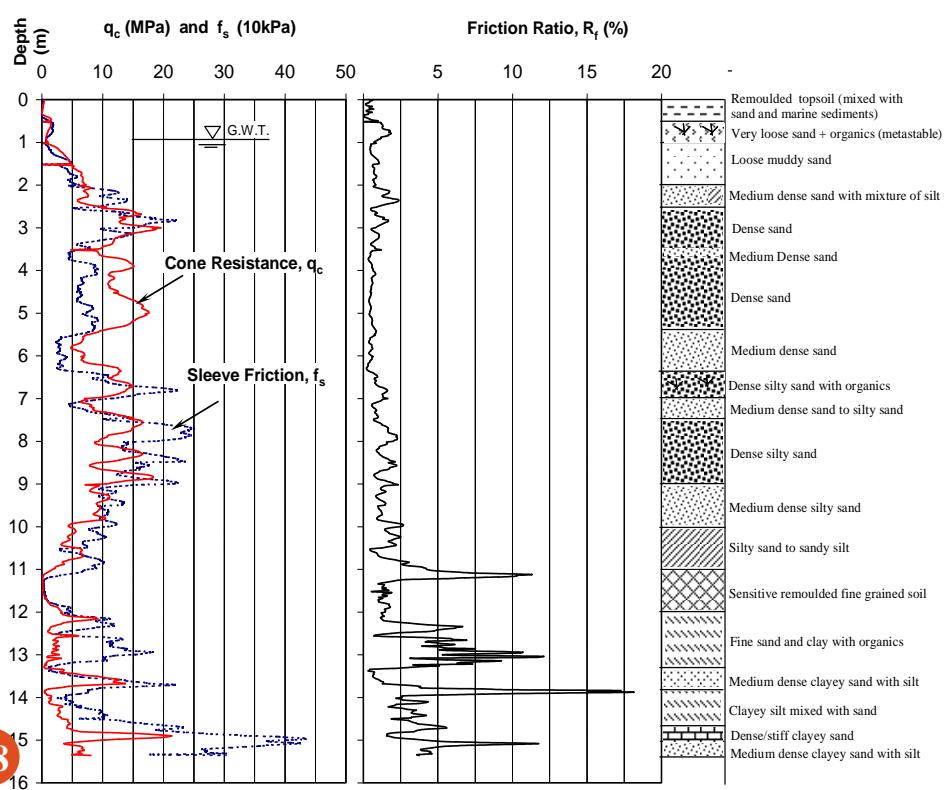
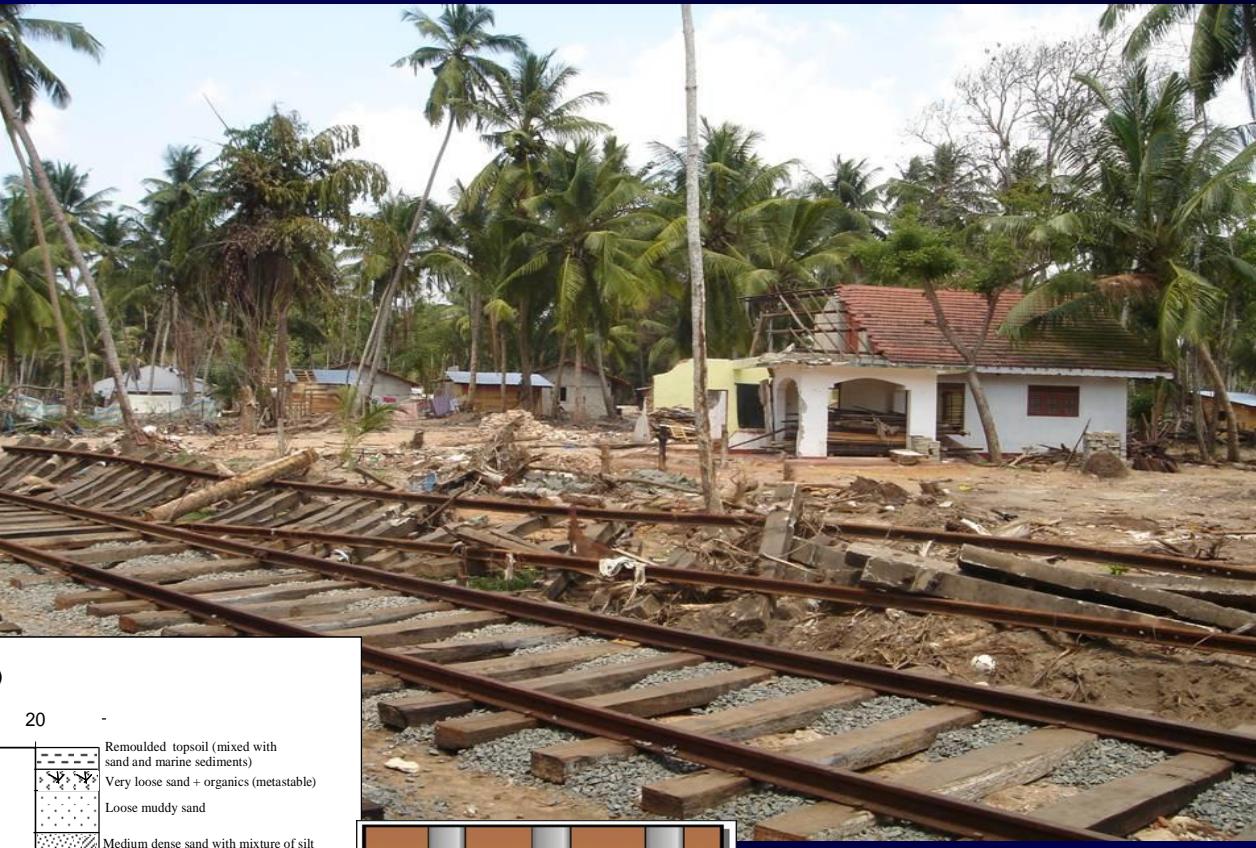


## Stressful Challenges in Disaster areas: Post-Tsunami Rail Track Rehabilitation in Sri Lanka (Jan-March 2005)



**Cone Penetration Test and Trial Pit**  
Mixed metastable sands with fine sediments (organic) at shallow depths followed by undisturbed coarser sand at greater depths

# Ground condition assessment using cone penetration test (CPT)



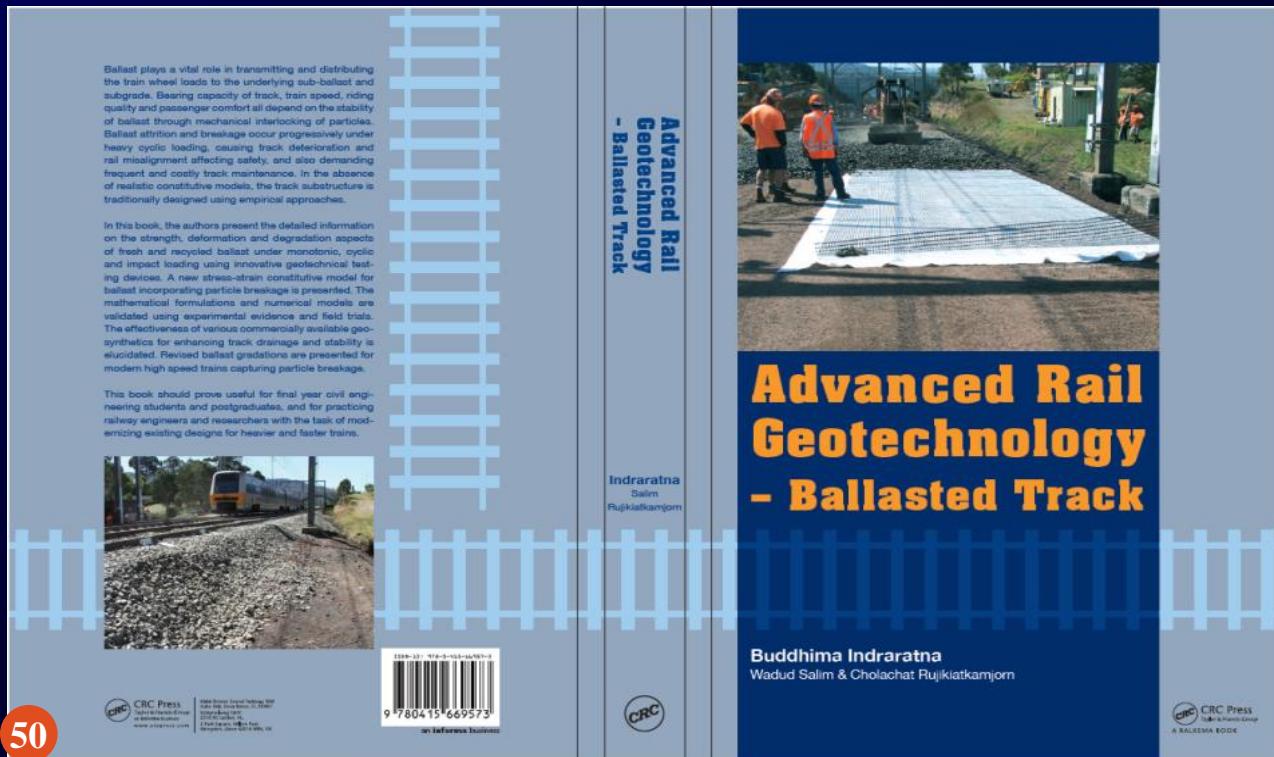
Observe the coconut palm trees !

# Conclusions and Recommendations

- Geosynthetics: increase internal **confining pressure** and **reduce particle movement** and **breakage** at elevated train speeds.
- **Computational FEM & DEM models** to predict track **degradation** with time, (c.f. empirical assessments).
- **Energy Absorbing Shock Mats** for minimizing impact damage
- **Application of PVDs** for improving soft subgrade soils and prevent mud pumping
- **Condition Monitoring via Field trials:** insight to complex track behaviour - performance verification.
- **Native Vegetation** – Green Corridors provide increased subgrade shear strength and less settlement
- **Ground Penetration Radar** can identify potential “adverse patterns” of hazards in track.

# Acknowledgment

- Australian Research Council (ARC)
- ISSMGE-TC202: Transport Geotechnics
- Centre for Geomechanics and Railway Engineering, University of Wollongong, Australia
- Past and Present research students, Research Associates and Technical Staff
- Industry Organisations: RailCorp (NSW), ARTC, QLD Rail, ARUP, Coffey Geotechnics, Douglas Partners. Roads & Traffic Authority, QLD Main Roads, Port of Brisbane Corporation, Port Kembla Port Corporation



Thank You!